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Original Citation:

Andalusian Timber Roof Structure in Chefchaouen, Northern Morocco: Construction Technique and Structural Behavior / Galassi, Stefano; Dipasquale, Letizia; Ruggieri, Nicola; Tempesta, Giacomo. - In: JOURNAL OF ARCHITECTURAL ENGINEERING. - ISSN 1076-0431. - ELETTRONICO. - 24:(2018), pp. 1-17. [10.1061/(ASCE)AE.1943-5568.0000315]

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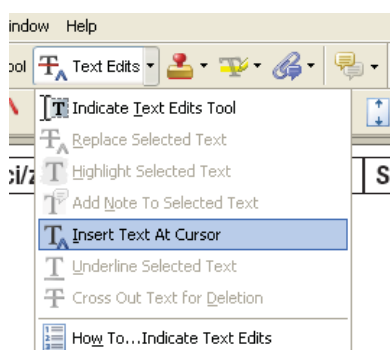
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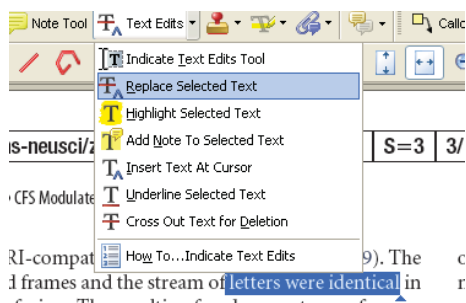
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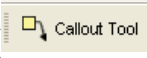
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Subject	Target contrast (%)
S1	12
S2	12
S3	15
S4	20
Mean \pm SEM	14.75 \pm 1.89

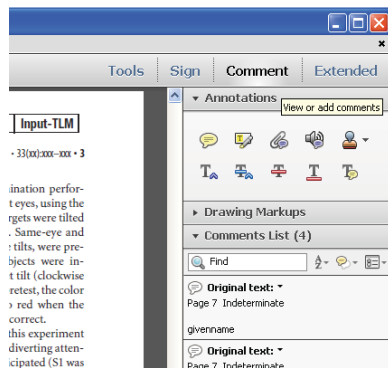
Each row corresponds to a different subject. Bottom row, mean and SEM across performance for target and mask presented to different eyes; well above chance.

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Andalusian Timber Roof Structure in Chefchaouen, Northern Morocco: Construction Technique and Structural Behavior

Stefano Galassi¹; Letizia Dipasquale²; Nicola Ruggieri³; and Giacomo Tempesta⁴

Abstract: This article presents the results of an investigation on the building system of the Andalusian timber roof, which is widespread in northern Morocco. The structural behavior of the Andalusian timber roof structures surveyed in the medina of Chefchaouen is analyzed in depth. The analysis, carried out using finite-element models, allowed for assessment of the structural behavior of the structure but also highlighted some weaknesses that are inherent to this building system. These weaknesses are primarily due to the presence of unilateral connection elements that ensure efficiency only under specific stress conditions and also to the lack of efficiency of the connection between load-bearing elements of the roof and the surrounding walls. The detected horizontal displacement of supports explains the cracking pattern that is usually visible at the top of walls just under the level of the gutter. A parametric analysis was performed, revealing that the weaknesses of the system do not present specific criticalities in the geographic context in which the system is developed. Nevertheless, some crucial strengthening interventions are proven to be necessary for ensuring that all timber elements can suitably contribute to the overall equilibrium of the structure in the case of an earthquake. DOI: [10.1061/\(ASCE\)AE.1943-5568.0000315](https://doi.org/10.1061/(ASCE)AE.1943-5568.0000315). © 2018 American Society of Civil Engineers.

Author keywords: Andalusian roof; Vernacular timber roof; Timber roof–masonry interaction; Règlement de construction parasismique RPS2000; Strengthening interventions; Seismic vulnerability assessment.

Introduction

Due to its refined and complex distinctive traits, noticeable in the geometric features of the structural elements, their organization, and the solutions adopted for the nodes, the Andalusian-type collar roof, widespread in the medina of Chefchaouen, has been the object of research, of both a historical and technological nature, and of surveys aimed at defining its construction traits (Dipasquale and Volpi 2009; Tampone 2001).

Important in-depth analysis was also carried out regarding the mechanical features of the walls of the buildings in which the Andalusian collar roof system is generally used (Rovero and Fratini 2013).

Rovero and Fratini (2013) identified three masonry types in the medina of Chefchaouen: MT1, MT2, and MT3. Type MT1 is a stone masonry made of hard limestone blocks, roughly hewn and irregular in shape. Some stone blocks running through the wall for approximately two-thirds of the thickness allow a certain transversal connection. Type MT2 is a three-headed load-bearing

brick masonry with usual block sizes of $21 \times 10 \times 2.5$ cm or $22 \times 10 \times 3$ cm, and the cross section of the wall is approximately 35 cm thick. Type MT3 is a mixed stone and brick masonry with a core of fine filling material and mortar in the wall section. With the aim of regularizing the wall structure and providing a connection between the internal and the external wall fabric, rows of bricks placed every 60–80 cm are generally present. In all masonry types, blocks are bound with a lime–earth mortar, executed by mixing a part of lime binder and a part of clay. The compressive strength of this mortar was evaluated to be approximately 25 N/mm^2 . Lastly, walls are protected by plaster, usually of earth and lime, and painted in a thousand shades of indigo. This practice demonstrates the concern for ensuring maintenance and adequate protection of the earthy mixture against rain, without which the whole masonry system would be subjected to decay.

In this study, numerical investigations were carried out to deepen the knowledge of the overall structural behavior of the constructive typology of the Andalusian timber roof. The analyses both highlighted some inherent critical elements due to the adopted technological solutions and allowed the assessment of its vulnerabilities, not only regarding gravitational loads but also with respect to seismic actions (Parisi et al. 2011; Parisi and Chesi 2014; Ruggieri et al. 2018). The results allow for the provision of targeted solutions for conservation and safeguard.

The medina of Chefchaouen, situated in the north of Morocco, was founded by the Andalusian Arabs in 1471, who chose to build a fortified city in a strategic position to defend the region from the Portuguese invasion, not far from the source of the Ras el Maâ River. Chefchaouen had its greatest period of development in the sixteenth and seventeenth centuries as a result of the fall of the kingdom of Granada in Spain (1492), which caused the incoming of large numbers of Arab Andalusian refugees who settled in this area, attracted by the fertility of the land and its strategic position (Dipasquale et al. 2008).

It is precisely due to the influence of the Spanish Andalusian culture, and to the fortunate integration with local Berber and Islamic

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Note. This manuscript was submitted on July 31, 2017; approved on January 30, 2018; published online on [REDACTED]. Discussion period open until [REDACTED]; separate discussions must be submitted for individual papers. This paper is part of the *Journal of Architectural Engineering*, © ASCE, ISSN 1076-0431.

traditions, that the medina of Chefchaouen efficiently represents the process of development of an architectural and urban culture with original and particularly significant traits.

The most recurrent traditional roofs in the medina of Chefchaouen are of the double-pitched type. These roofs, as can be deduced from observation, in-field surveys, and interviews with local master builders (*maâlem*), are classifiable into two structural categories: the Berber structure and the Andalusian structure.

The former (Fig. 1) is the simplest and also the oldest one. The ridge beam is supported by a bearing structure composed of very close sloping rafters. The pitch slope is between 30 and 40%. In the case of wide spans to be covered (more than 5 m), the Berber system is aided by additional elements, namely, two principal rafters joined to a horizontal beam. The king post is directly joined to the tie-beam, constituting a not-beneficial concentrated load for the tense element. Hence, this configuration makes the static behavior of this constructive typology ineffective, which indeed always shows very deformed structural elements. The behavior of the Berber structure is currently under analysis, and the results of the investigation will be provided in a following paper.

The Andalusian structure is more complex and interesting from the constructive point of view; it uses well-squared and often finely decorated wooden elements (Dipasquale and Volpi 2009). The most widely used wood species in Chefchaouen, all from local sources, are cedar (*Ærz*), fir (*soha*), and red fir (*sanawbar*). Whereas cedar is often used for decorations, fir and red fir are mainly used to build roof structures.

A key element in Moorish architecture, the Andalusian roof is a recurrent building system in central and southern Spain (Anderson and Rosser-Owen 2007), where it is called *armadura de par y nudillo*, meaning a structure of rafters (*pares*) and collar beams (*nudillos*). During the period of Arab domination (711–1492), this region was known as Al-Andalus and received architectural and artistic influence from the Muslim culture and from the North African Berbers and the classical Roman tradition already present in Andalusia. In this area, it is still possible to find examples of *armadura de par y nudillo*, especially in religious buildings converted into churches and in noble palaces (Nuere 1989; Candelas Gutiérrez 2003). In the medina of Chefchaouen, the Andalusian-type wooden structure was imported by the Andalusian master builders who settled there and was widely used—with substantial modifications regarding the constructive technique mostly used in Spain, which are not noticeable at first sight—for the roofs of the rooms of the courtyard house and the bays of mosques. A very similar structural organization is also emphasized in many church roofs in the Sicily region (Copani 2006). An eminent example is the Nicosia cathedral (Catania) that dates back to the fifteenth century and derived from the Arab domination (ninth to eleventh centuries) and consequent constructive culture influence on the Sicily region (Tampone 2005).

The article is organized as follows: The constructive system of the Andalusian timber roof structure is described in the second

section, and a fundamental comparison between the Moroccan and the Spanish Andalusian version is also provided. The third section is devoted to the reference case study of an ancient courtyard house in the medina of Chefchaouen, in which the structure type under analysis is found. The role of each timber element is investigated, and the structural behavior is assessed. In the same section, the results of the analyses, which highlight some inherent weaknesses of this type of structure, are summarized. The results explain and are coherent with the external cracking pattern detected. The fourth section deals with the seismic behavior assessment of the structure, taking into account, as a reference, the provisions in the local regulations. The final section presents concluding remarks.

Construction Analysis

The structure of the Andalusian-type roof is constituted by a single-frame double-pitched roof, made with the use of coupled rafters placed with a slope of approximately 85% and oriented in accordance with the shorter side of the room to be covered. The span varies from 3 to 4 m, in courtyard houses, up to 7 m, in the case of mosques.

The coupled rafters are counterposed and connected at the ridge with the use of a plank, which is particularly useful during the phase of setting the structure with the aim of maintaining the spacing established for placing the other elements. Therefore, the roof carpentry work does not include an actual ridge beam. Every coupled rafter includes a transversal connection timber element (collar beam) with a section of 5–7 by 10 to 15 cm and a length of approximately 60 to 65 cm, whose far ends are adequately shaped and carved so as to provide suitable support for the connecting joints (Fig. 2).

The collar beams, placed in the upper part of the structure at a distance from the ridge of about one-fifth of the rafters' length, support wooden planks that, with their thickness, are wedged into the grooves carved in the rafters. In that same spot, additional planks completing the connecting system are placed, orthogonally arranged to the roof surface and wedged into the corresponding grooves in the collar beams [Figs. 3(a and c)].

The central part of the roof presents an additional set of boards at the lower edge of the collar beams, wedged to the collar beams and nailed to the boards [Fig. 3(b)]. The ensemble of these elements constitutes the *bsat* (Dipasquale and Volpi 2009).

From the geometric and constructive features of the joint between the collar beam and the rafter, it can be noticed how that device provides only a unilateral connection, capable of transferring compression forces but inefficient if subjected to tensile forces [Figs. 3(b and c)]. In the constructive technique of the aforementioned structural system, the boards that constitute the external deck of the roof, which are directly nailed to the rafters, assure a good overall stiffness and, at the same time, nullify the tendency to stack the individual structural units, preserving the spacing between them unchanged (Tampone and Ruggieri 2016).

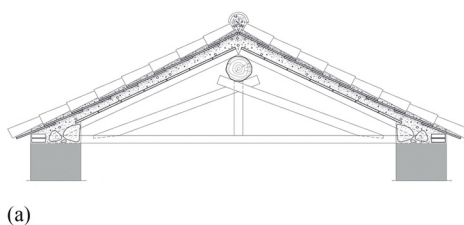


Fig. 1. (a) Sectional elevation; (b) Berber roof structure (image by Letizia Dipasquale).

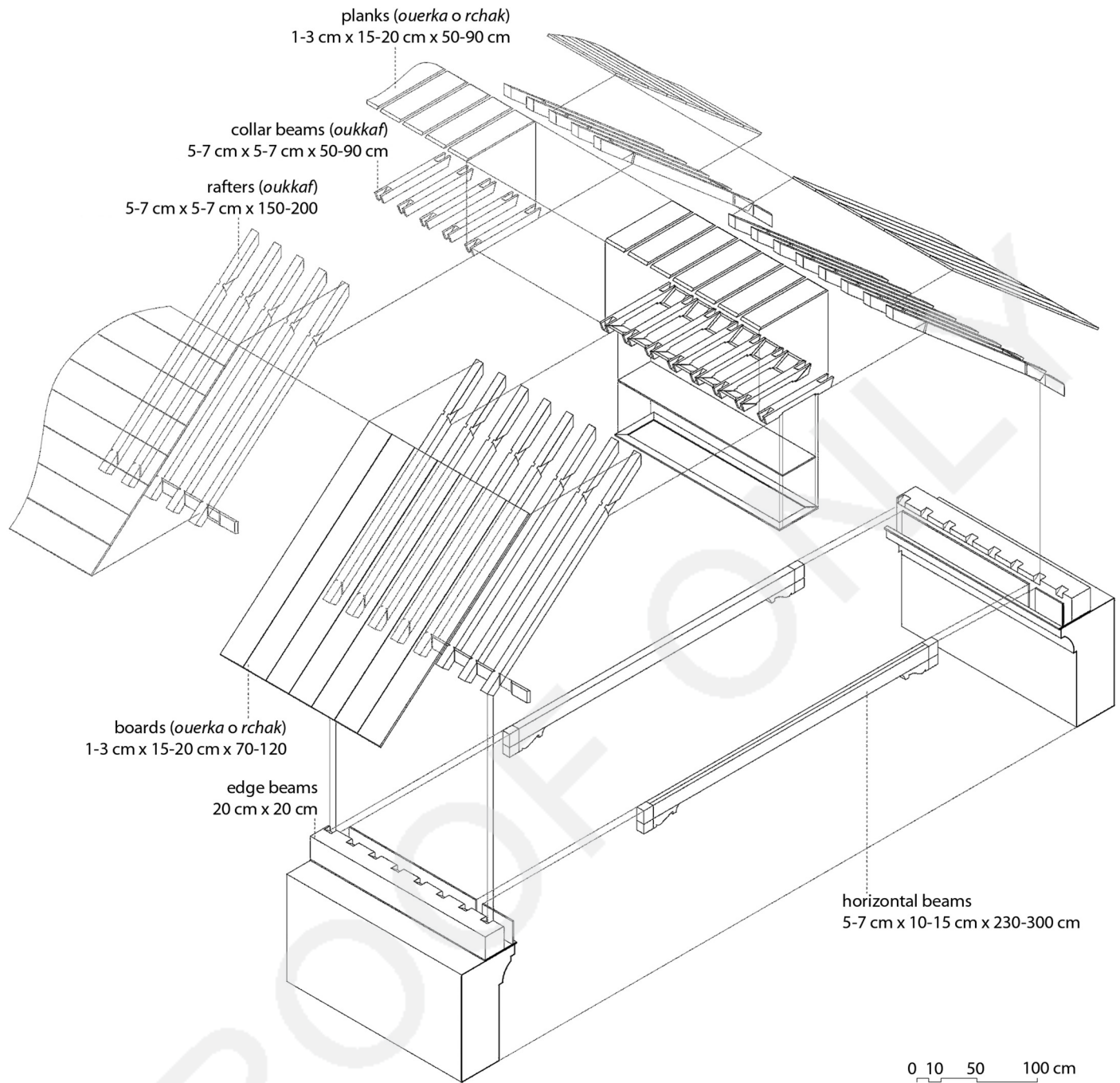


Fig. 2. Exploded view drawing of the Andalusian-style roof: Procedure of assembly of the elements.

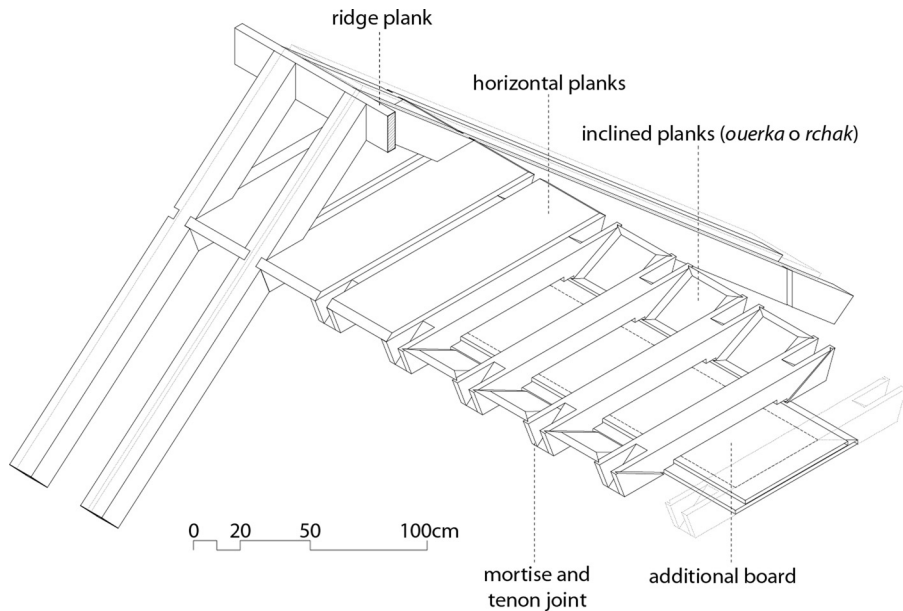
F2 : 1

170 The bases of each couple of rafters are connected to two edge
 171 beams with the rectangular cross section, placed at the top of the
 172 longitudinal walls, which provide support for the roof structure and
 173 allow a good distribution of the actions transmitted on the masonry
 174 walls. The connection between the main elements of the roof and the
 175 edge beams is obtained through a half-lap joint, that is, a cavity
 176 hollowed out on the upper corner of the edge beams, which holds
 177 the head of the rafter [Fig. 3(d)]. The edge beam thus has a square
 178 and beveled section alternately. The general carpentry elements are
 179 presented in Table 1.

180 With the purpose of providing the roof deck for both pitches,
 181 wooden boards are nailed to the upper edge of a couple of rafters,
 182 arranged transversally to the room at the center and longitudinally
 183 at the ends (Fig. 2).

184 The deck provides support for the screed and for the roofing
 185 tiles. The screed, with a thickness between 5 and 12 cm, is consti-
 186 tuted by a mix of earth, lime, pebbles, and fragments of bricks. The
 187 curved tiles (which are made of a mix of clay, sand, straw, and or-
 188 ganic elements, molded by hand and baked in traditional wood
 189 ovens) are placed in two superposed inverted layers directly on it.
 190 Fig. 4 shows the covering and the arrangement of the tiles to provide
 191 two typical gutter systems, the simple and the protruding gutter.
 192 These systems are aimed at directing the rainwater away from the
 193 masonry walls and preserving the earth mortar and plaster.

194 Additional horizontal beams (Fig. 5) are often placed at the level
 195 of the edge beams, with a rectangular cross section of approxi-
 196 mately 7 by 15 cm, arranged transversally to the room without a
 197 structural connection with the edge beams, and are thus unable to



(a)

(b)



(c)

(d)

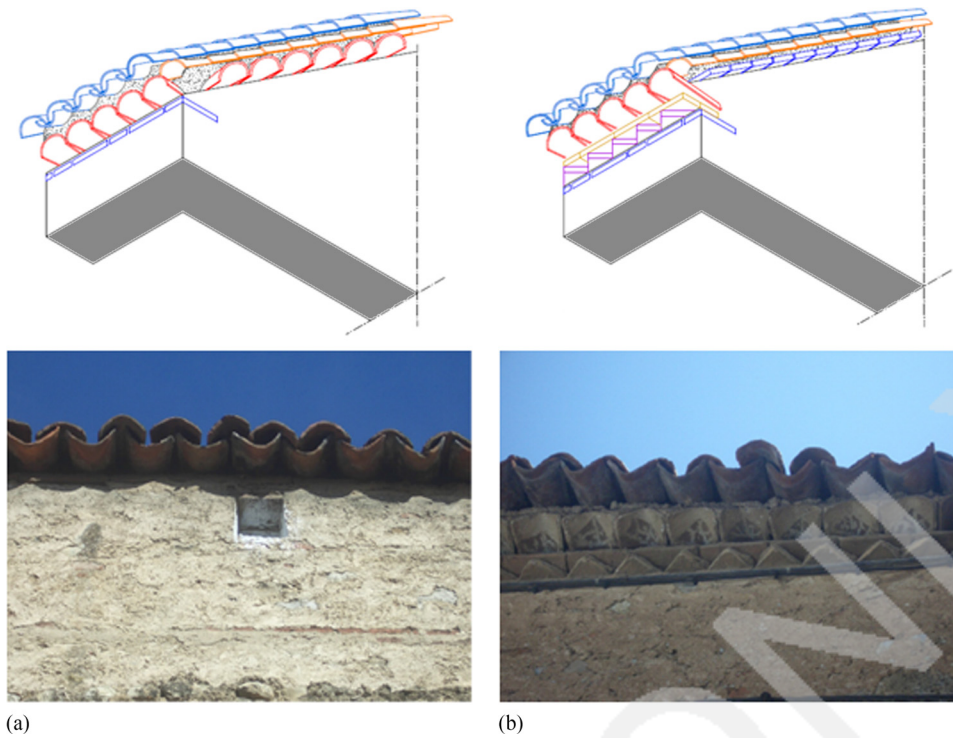
Fig. 3. (a) Schematic representation of the longitudinal connection of the structural system's coupled rafters-collared beam with the use of inclined and horizontal planks placed into grooves carved on the heads of the collar beam and in the tenons on the rafters; (b) set of boards at the lower edge of the collar beams; (c) detail of the mortise joint on the collar beams; (d) detail of the node between rafter and edge beam. [Images (b-d) by Letizia Dipasquale.]

Table 1. Dimensions of the carpentry elements

Wooden elements	Size (cm by cm)
Rafter (<i>Oukkaf</i>)	5-7 by 5-7
Collar beam	5-7 by 10-15
Ridge plank	4-5 by 10-12
Planks	1-3 by 15-20

198 provide contribution to bear the thrust from the roof structure,
199 unlike a traditional king post roof truss or the *par y nudillo* timber
200 roof in Spain, from which the Moroccan version was derived. In
201 fact, in the Spanish Andalusian system (Nuere 1989), the transversal
202 beams can behave as tie-beams, given their U-shaped grooves
203 where the longitudinal edge beams are placed, forming a crosslap

204 joint (compare Figs. 2 and 5 with Fig. 6). Instead, in the Moroccan
205 version, the transversal beams do not work as actual ties because
206 the connection with the edge beams is not sufficient to assure such a
207 role because the link, which is not present in all cases, is made of
208 simple metal brackets or nails with a wooden corbel. Considering
209 this node geometry, although a pair of nails is usually present, the
210 connection cannot transfer the tensile force from the transversal
211 beams to the edge beams. These beams, usually placed in couples
212 [Figs. 5(a and d)], with a variable spacing, usually alternate, of
213 approximately 1.30 and 0.45 m, have only the function of providing
214 support for a possible attic, usually used as a storeroom or garret
215 [Fig. 5(b)]. With the aim of reducing the span of these beams and
216 providing a suitable end support, the walls include a series of bricks
217 that protrude approximately 15 cm, with the addition of the afore-
218 mentioned wooden corbel.

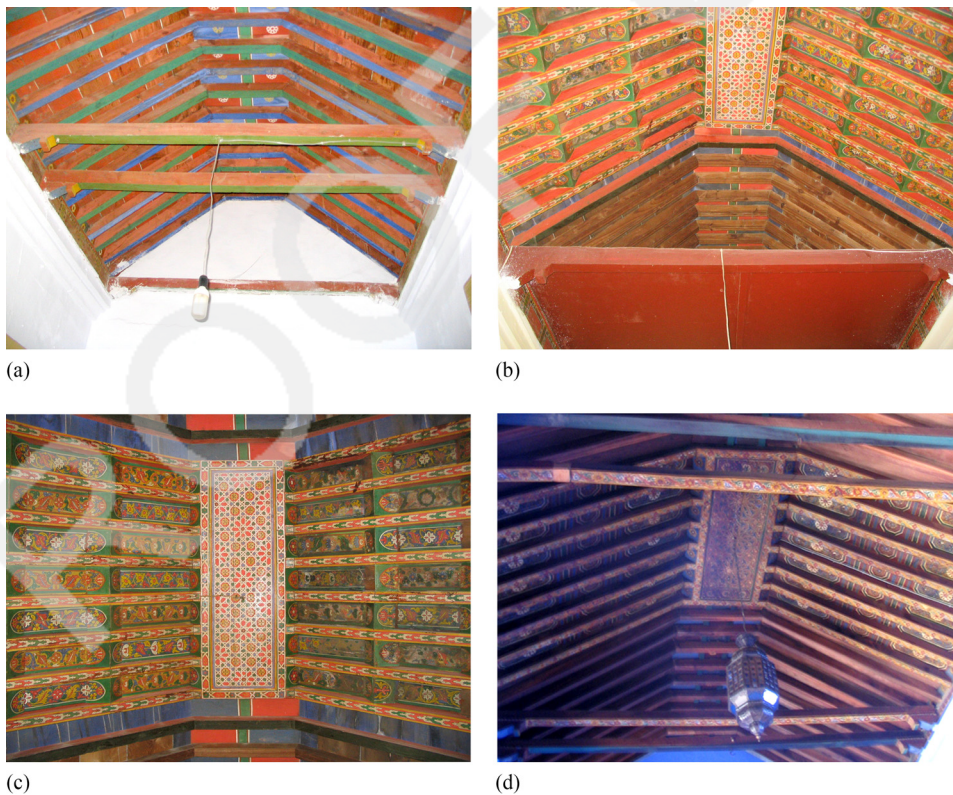


(a)

(b)

Fig. 4. Gutter systems: (a) simple gutter; (b) protruding gutter. (Images by Letizia Dipasquale.)

F4 : 1



(a)

(b)

(c)

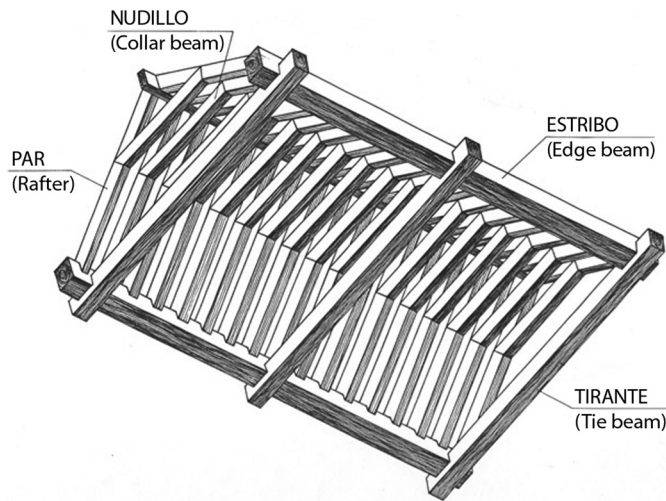
(d)

Fig. 5. Internal view of an Andalusian-type roof with floral and arabesque decorations: (a) without mezzanine attic; (b) with mezzanine attic; (c) details of the colored decorations; (d) details of the transversal beams. (Images by Letizia Dipasquale.)

F5 : 1
F5 : 2

219 The system is characterized by great simplicity of execution. In
220 fact, retracing the main construction phases, first the edge beams
221 are placed on the side walls, then each pair of rafters is erected and

placed together with the transversal connecting element. The base 222
of the rafters is joined to the edge beams by way of the notch 223
carved into the edge beams themselves, without nails. The next pair could 224



F6 : 1 **Fig. 6.** Andalusian-type roof in Spain (with tie-beam elements) known
 F6 : 2 as *armadura de par y nudillo*.

225 be placed after the insertion of both the inclined and horizontal
 226 planks, which connect them to the previous pair. The spacing
 227 between the rafters, very reduced (between 10 and 20 cm), corre-
 228 sponds approximately to double their width. The building proced-
 229 ure just described is very similar to a sort of modern prefabrica-
 230 tion system. The site was always headed by a *maâlem* (i.e.,
 231 master builder), who had an excellent knowledge of geometry and
 232 was capable of drawing and representing, by in-scale mod-
 233 els, the structure of the roof to be built. It was the *maâlem* himself
 234 who established the dimension and spacing of the elements,
 235 based on the size of the room and the slope of the roof.

236 The inner surface of the Andalusian-style roof is often painted
 237 with geometric, floral, and arabesque patterns and framed by addi-
 238 tional wooden planks [Figs. 5(b-d)].

239 Case Study

240 With the purpose of assessing the safety of the structural system
 241 used in the configuration of the Andalusian roof and in attempt to
 242 highlight its vulnerabilities (Cruz et al. 2015), both inherent and
 243 deriving from possible seismic events, a series of numerical simula-
 244 tions was carried out with the finite-element method (FEM) soft-
 245 ware Straus7. The reference case study of the Raissouni *dar* was
 246 examined, which is the oldest courtyard house in the medina of
 247 Chefchaouen. Built by the founder of the city, Moulay Ali Ben
 248 Rachid, the house underwent several transformations throughout
 249 the years, and today the building represents an example in which
 250 the constructive solutions adopted are among the most refined, tech-
 251 nologically more advanced and structurally more correct. In partic-
 252 ular, the roof analyzed is that of the *ghorfa* (bedroom), the common
 253 space in which the family nucleus spends most of its domestic life.

254 Dimensions of both masonry walls and roof timber elements
 255 were ascertained by on-site inspections during surveys. The covered
 256 room is an 8.60 × 2.90 m rectangular space with 0.35-m-thick walls
 257 in stone and bricks set in a mortar of mixed lime and earth.

258 The load-bearing structure of the roof, arranged according to the
 259 typical Andalusian configuration, is constituted by the usual
 260 sequence of counterposed couples of rafters set with a slope of
 261 72.5% (approximately 40°) and placed with a spacing of 0.215 m
 262 (Fig. 7).

263 The mechanical and dimensional characteristics of the elements
 264 that form the structure, assumed in the numerical models, are pre-
 265 sented in Table 2 and Fig. 7. Because experimental data were not
 266 available for the mechanical features, reference was made to con-
 267 ventional values from the standard UNI 11035-3:2010 (UNI 2010).
 268 These values, and in particular the specific weight, coincide with
 269 those provided by Eurocode 1 (CEN 2002) relative to the timber
 270 strength class C18 reported in the standard UNI EN 338:2009 (UNI
 271 2009), which are obtained for timber at a moisture content consist-
 272 ent with a temperature of 20°C and a relative humidity of 65%.
 273 Such values were assumed in this study because they are coherent
 274 with the typical Mediterranean climate of Chefchaouen, character-
 275 ized by high humidity and quite high temperatures (the mean tem-
 276 perature is equal to approximately 17°C).

277 In the structural model, the transversal beams that provide sup-
 278 port for the attic deck were considered due to the effective lack of
 279 connection with the edge beams of the roof detected.

280 Table 3 shows the incidence of self-weight loads acting on each
 281 rafter, listed both per unit area and as load uniformly distributed on
 282 the axis of the element.

283 Structural Behavior Assessment of the Timber Elements

284 Because the roof structure is generated by a repetitive elemental
 285 frame, composed of a couple of sloping rafters and the collar beam,
 286 a fundamental investigation to detect the exact function of each tim-
 287 ber element was carried out by the analysis of very simple two-
 288 dimensional (2D) models. Three configurations of this model were
 289 conceived, where only the external supports were changed. All geo-
 290 metric and dimensional features of the elements were preserved, as
 291 were the rafter-to-rafter and rafter-to-collar beam links, assumed as
 292 internal hinges. The collar beam was considered as a truss, capable
 293 of transmitting the axial load but also subjected to the bending
 294 moment due to its self-weight.

295 Through direct site inspections, it was ascertained that the roof
 296 system is simply supported on the room walls, by means of the edge
 297 beams, without fasteners. Based on this observation, a first model
 298 was conceived (Model FEM_1) as supported on a roller and a
 299 pinned support, respectively. As expected, in this model the collar
 300 beam is necessarily subjected to a tensile force, taking the role of
 301 the lacking tie-beam at the base of the system. However, it is not
 302 coherent with the technological solution adopted in the tenon and
 303 mortise joints between the collar beam and the coupled rafters
 304 (Parisi and Piazza 2000); therefore, the boundary conditions of this
 305 model cannot effectively describe the behavior of the actual struc-
 306 ture. In Fig. 8, the results of the analysis are shown; specifically, it
 307 is worth noting the following:

- 308 1. The structure transfers to the walls an exclusively vertical
 309 action equal to 0.73 kN.
- 310 2. The tensile force in the collar beam, equal to 2.07 kN, is cer-
 311 tainly not negligible.
- 312 3. The very high horizontal displacement of the roller support
 313 equal to 4.40 cm directed toward the exterior of the room,
 314 which is also lowered by the chain effect of the collar beam
 315 according to the assumed hypotheses of this model, actually
 316 would detach the collar beam from the couple of rafters and
 317 transform the structural system into a collapse mechanism due
 318 to the spreading of the two pitches.

319 Therefore, from the considerations summarized in the previous
 320 list, it can be easily deduced that the structure was originally
 321 intended by the Moroccan master builders (*maâlem*) as a statically
 322 indeterminate system whose supports on the walls work as two
 323 pinned joints. Furthermore, because the two supports of the roof to
 324 the walls are of the same type, it should not be correct to assume

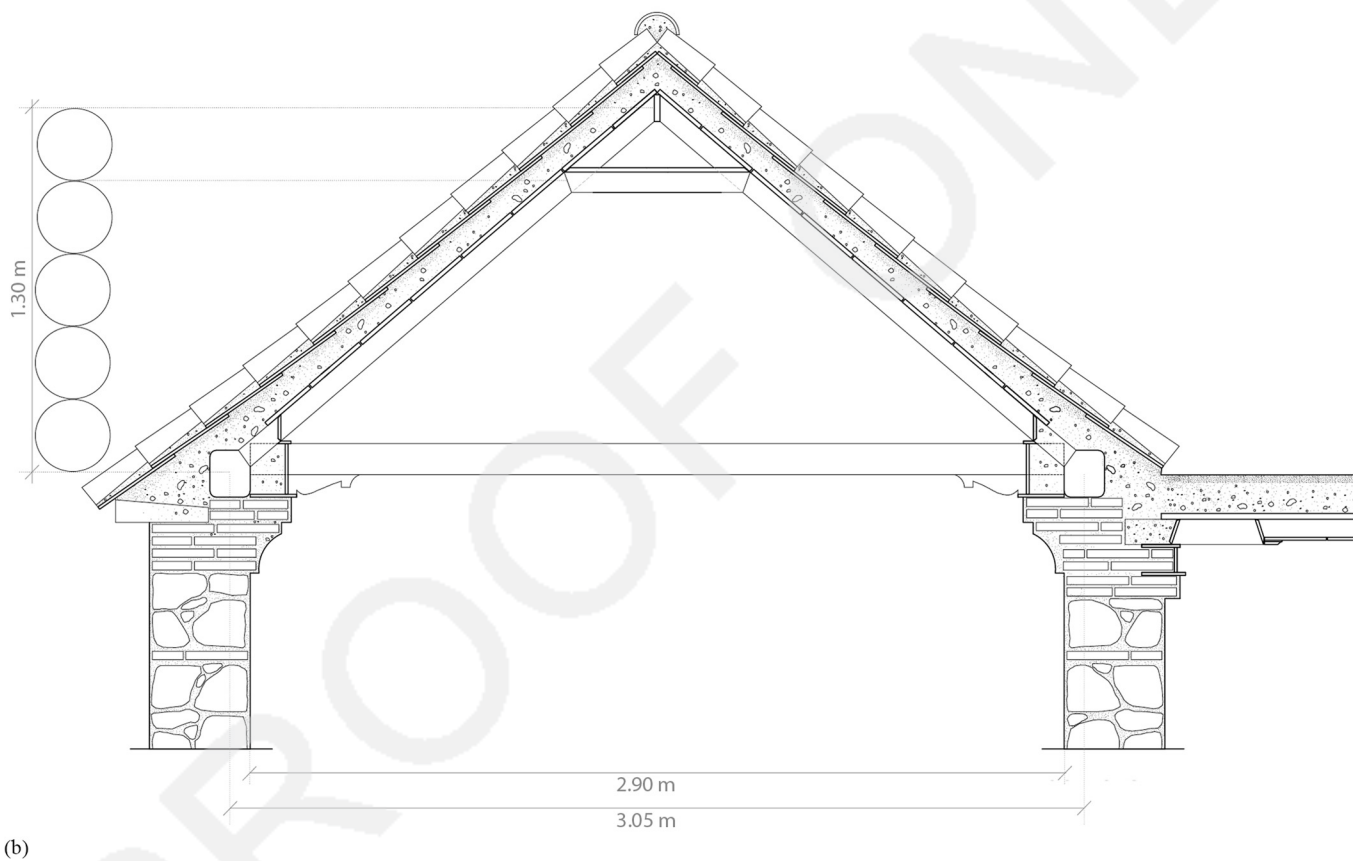
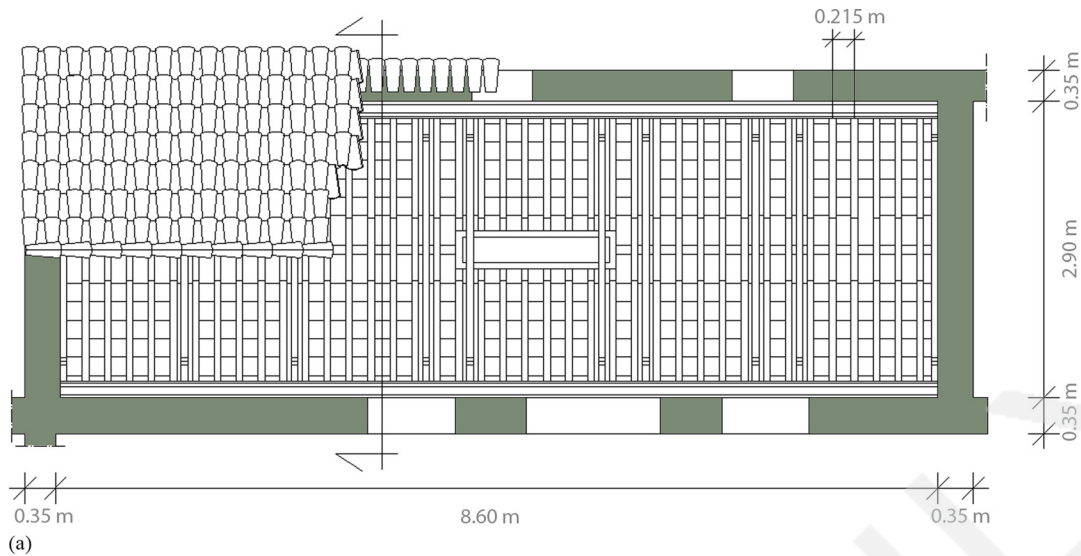


Fig. 7. *Ghorfa* roof structure in the Raissouni *dar*: (a) plan; (b) transverse section.

F7 : 1

Table 2. Mechanical and dimensional characteristics assumed for the timber elements of the roof structure

FIR elements	Size (cm by cm)	Specific weight (kN·m ⁻³)	Elastic modulus (N/mm ²)	Flexural strength (N/mm ²)
Ridge plank	5 by 10	3.8	7,200	28
Rafter	6.5 by 7.2			
Collar beam	6.5 by 7.2			
Edge beam	14.5 by 16.5			

Source: Data from CEN (2002); UNI (2009, 2010).

Table 3. Self-weight load acting on each rafter

Roof floor	Description	Thickness (cm)	Incidence of load per unit area (kN·m ²)	Uniformly distributed load (kN·m)
Deck planking	Wooden boards	1.5	0.057	0.012
Screed	Mix of earth, lime, pebbles, and fragments of bricks	8	0.96	0.21
Covering tiles	Mix of clay, sand, straw, and organic elements, molded by hand	—	0.6	0.13

one as a hinge and the other as a roller. Accordingly, a second model (FEM_2) was made that considers these supports (Fig. 9).

The analysis of this second model underlined that the collar beam behaves as a strut under a compression force equal to 0.75 kN, in agreement with the technological solution adopted for building the joint under study, and that the structure transfers to the walls a horizontal thrust equal to 0.59 kN and a vertical action of 0.73 kN.

Furthermore, the stress state of the structural elements was found to vary in a very reduced range, between -1.33 and $+1.22$ N/mm², values that are much lower than the limit values of the fir wood. The most stressed areas were detected at the midspan of the rafter and near the joint with the collar beam. The maximum vertical displacement, detected at the midspan of each rafter, was equal to 0.05 cm, a value that is much lower than the limit value (0.53 cm, i.e., 1/300 of the span). Therefore, in this model the strength and serviceability verifications are also satisfied.

According to the results just presented, the behavior of the second model could match the actual behavior of the real structure. However, it is necessary to note that the thrust provoked by the rafters on the masonry walls must rely only on the friction reaction that is produced in the timber–masonry interface, between the edge beam and the wall, due to the lack of the tie-beam at the base of the roof system.

A shear-sliding verification was, therefore, carried out at the timber–masonry interface. The criterion of Coulomb's friction cone was adopted, and the value of the static coefficient of friction was taken from the technical literature.

Because the value of the actual friction coefficient was not available, reference was made to values from the technical literature, which range from 0.4 to 0.7 (Du Bois 1902; Mastrodicasa 1948; Murase 1984; Blau 1996; Grigoriev and Meilikhov 1997; Elert 2017; Gorst et al. 2003; Lee et al. 2005). The highest value of 0.7 was chosen as the reference value to assess if the roof structure had the propensity to slip on the walls already in the optimal condition due to the highest friction reaction.

The computed shear force (i.e., the horizontal thrust transmitted by the structure of the roof to the wall), equal to 0.59 kN, is slightly higher than the maximum value of the friction reaction that the beam–wall joint can exert ($f \cdot N = 0.7 \times 0.73 = 0.51$ kN). Therefore, this condition would highlight the propensity of the structure to suffer a horizontal displacement exactly in correspondence to the edge beam.

To deepen this phenomenon, an additional analysis was carried out in a further model (FEM_3), simulating the presence of a horizontal inelastic displacement at the level of the edge beams.

The third model (Fig. 10) was analyzed using a step-by-step procedure of increasing inelastic displacements applied to the external supports (Galassi et al. 2013; Orlando et al. 2016). The process was interrupted at the step in which the axial load in the collar beam became positive (traction) and, therefore, incompatible with the actual performance of the element. The results of this parametric analysis are presented in Table 4. The values reported in Table 4 are also graphically represented in Fig. 11.

The diagrams presented in Fig. 11 highlight that, for a value of the inelastic displacement equal to 0.30 cm for both supports, the horizontal thrust transmitted by the structure on the edge beam is exactly equal to the friction reaction, which ensures the equilibrium

of the system. This means that, in correspondence to that value, the sliding of the edge beam toward the exterior stops, and the structure finds a new equilibrated configuration.

The last displacement considered in the parametric analysis (approximately 0.6 mm), at which the axial load in the collar beam changes sign, corresponds to the collapse of the structure, which transforms into a mechanism. The ultimate displacement detected is twice the displacement at which the thrust on the wall is balanced by the friction force. This highlights considerable safety in the case of gravitational loads.

Discussion of Results and Strengthening Interventions

The timber roof structure of the Raissouni *dar* represents a case study that is sufficiently representative of the Andalusian-type roof in Morocco.

The numerical analyses carried out highlighted the structural behavior of the timber structure in which a significant role is played by the collar beam that connects, in proximity of the ridge of the roof, the two counterposed rafters that form the covering surface. It was ascertained that, given the peculiar tenon and mortise joint, the horizontal beam must behave as a strut and contributes, on the one hand, to containing the flexural deformation of the roof and, on the other, confers a higher degree of safety to the efficiency of the hinge-joint between the two rafters near the cusp, especially in the presence of nonsymmetrical actions.

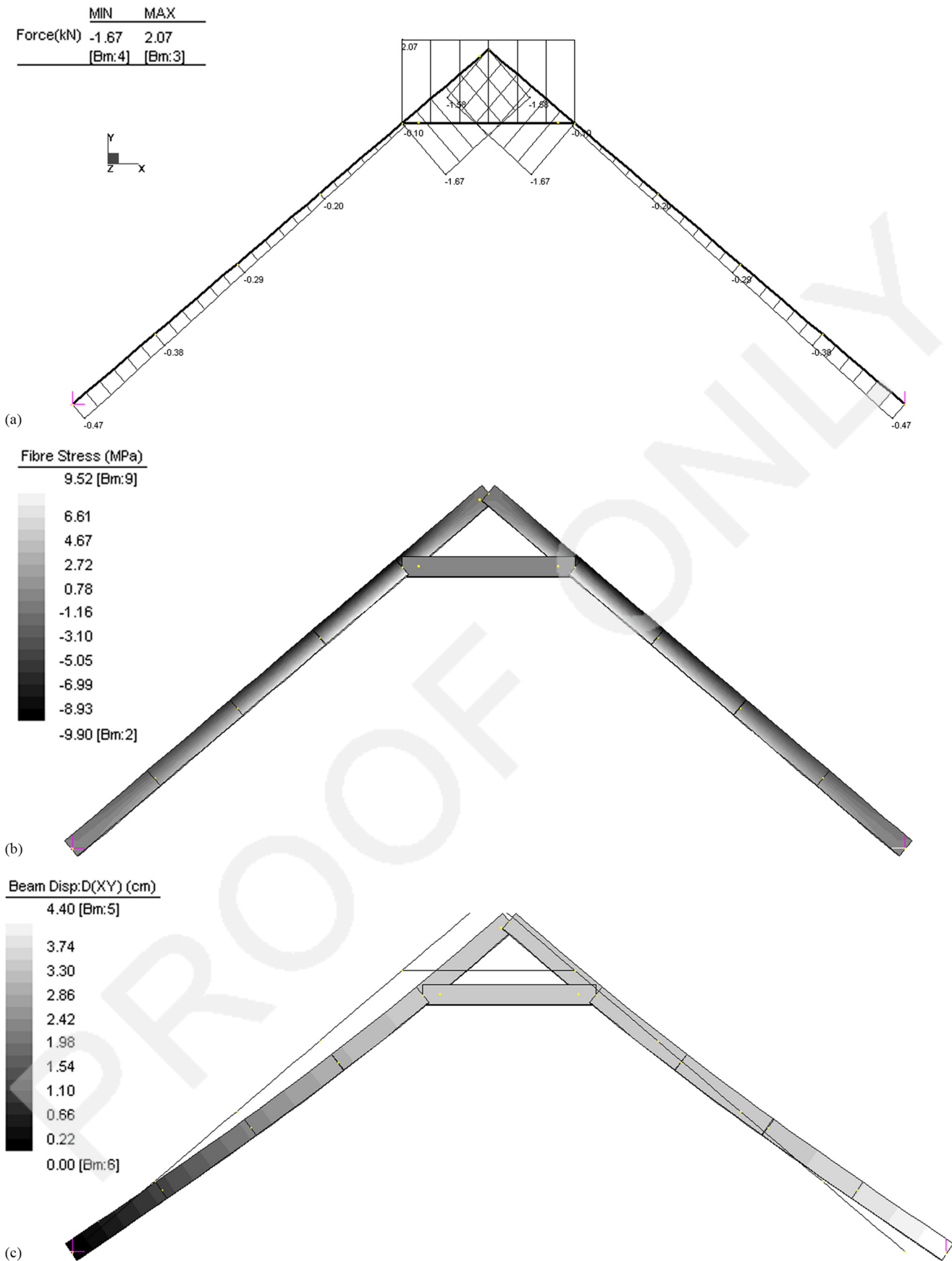
In detail, it was proven that the Moroccan-type Andalusian roof system shows a general structural consistency and a sufficient level of safety in the case of gravitational loads, even if the structure is in a state of unstable equilibrium ensured only by the friction between the edge beam and the masonry.

The main reasons can be listed as follows:

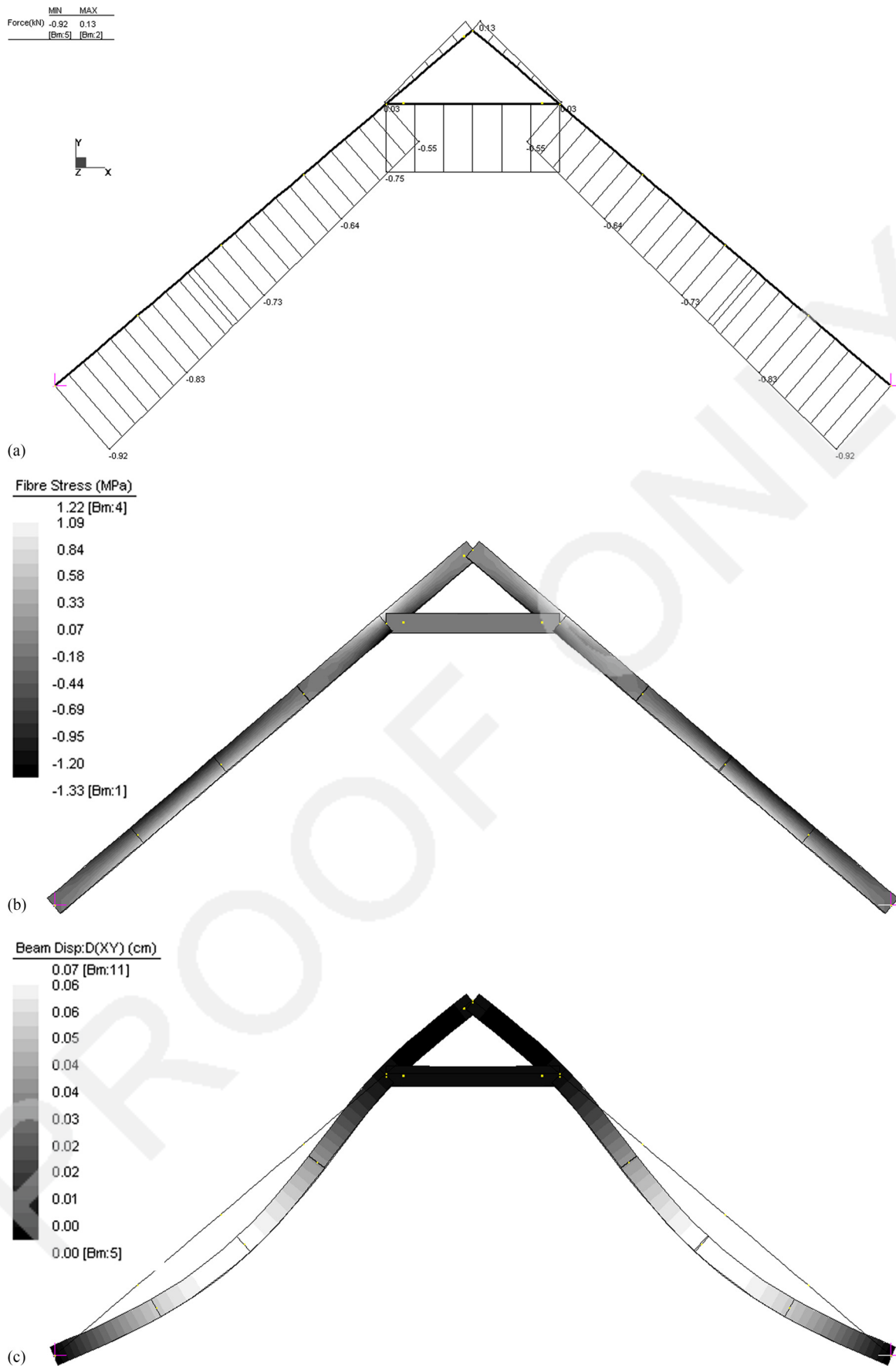
- The spacing between the rafters is moderate (21.5 cm).
- The high slope of the roof pitch and, therefore, of the elements that constitute the load-bearing structure (over 70%), together with the low incidence of the dead loads as a result of the moderate spacing between the rafters, determines a very low horizontal thrust on the wall.
- Given its location in Morocco, variable loads cannot reach significant values; snow, for example, is not a possible load condition. Thus, any increase over time in the stress on the load-bearing timber elements and of the thrust on the walls is not possible.

However, negative characteristics of the roof affecting the safety for gravitational loads and meaningful critical elements are identified as follows:

- The approximately 8- to 10-cm-thick screed over the planks is an extremely heavy load, but its distribution on each element does not reach very high values thanks to the reduced spacing.
- The tenon and mortise joint between the collar beam and the timber elements of the pitch acts as a unilateral connection.
- There is a lack of an actual ridge beam.
- The edge beams are simply supported on the masonry walls without fasteners.

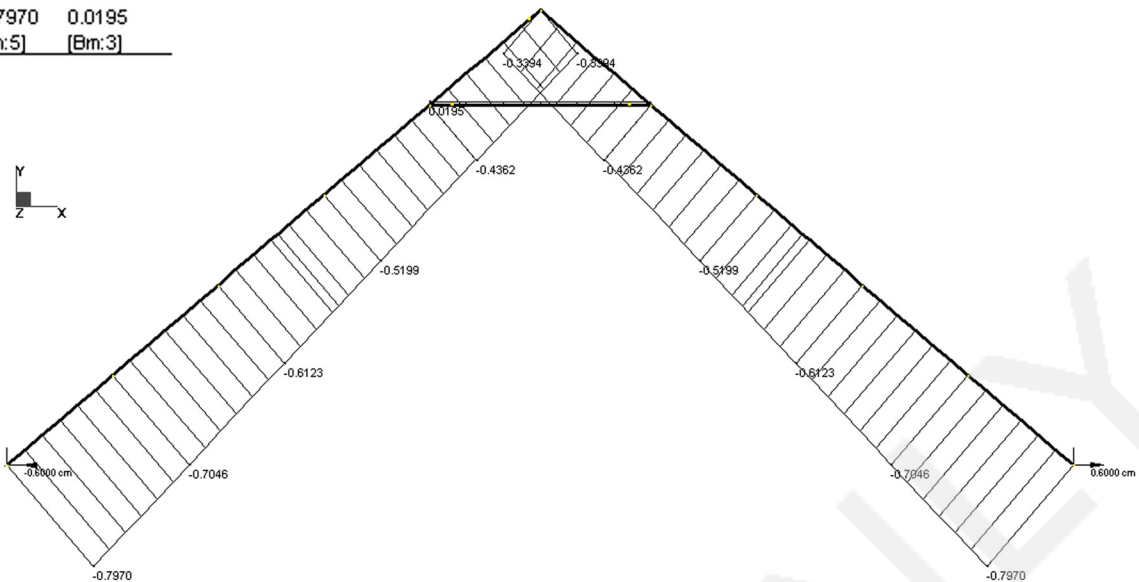


F8 : 1 **Fig. 8.** FEM_1 Model—statically determined structure joined to the walls by a pinned and a roller support: (a) axial load; (b) stress state; F8 : 2 (c) horizontal and vertical node displacements.



F9 : 1 **Fig. 9.** FEM_2 Model—statically indeterminate structure joined to the walls with pinned supports: (a) axial load; (b) stress state; (c) horizontal and
 F9 : 2 vertical node displacements.

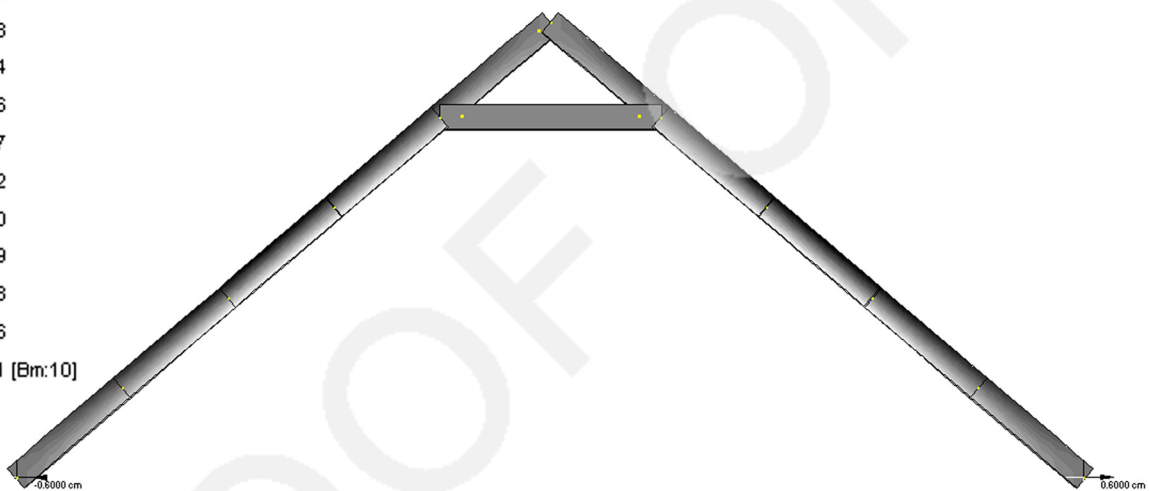
	MIN	MAX
Force(kN)	-0.7970	0.0195
	[Bm:5]	[Bm:3]



(a)

Fibre Stress (MPa)

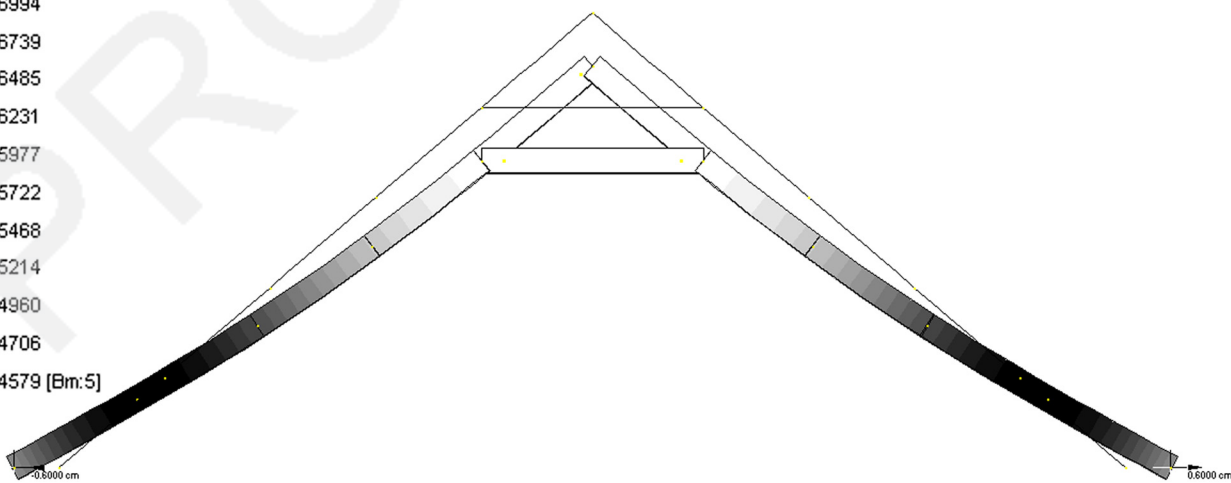
2.3946 [Bm:10]
2.1432
1.6403
1.1374
0.6346
0.1317
-0.3712
-0.8740
-1.3769
-1.8798
-2.3826
-2.6341 [Bm:10]



(b)

Beam Disp:D(XY) (cm)

0.7121 [Bm:2]
0.6994
0.6739
0.6485
0.6231
0.5977
0.5722
0.5468
0.5214
0.4960
0.4706
0.4579 [Bm:5]



(c)

F10 : 1 **Fig. 10.** FEM_3 Model—statically indeterminate structure joined to the walls by pinned supports subjected to inelastic displacements of approximately 0.6 cm: (a) axial load; (b) stress state; (c) horizontal and vertical node displacements.

F10 : 2

- The horizontal beams arranged transversally to the room are not connected to the longitudinal edge beams.
- There is a lack of actual tie-beams.
- The connection between each pair of rafters in the longitudinal direction relies only on horizontal and inclined planks wedged into grooves carved in the rafters and does not realize a perfectly three-dimensional (3D) structure behavior.
- The node between the rafters and the edge beams is obtained through a simple cavity hollowed out on the upper corner of the edge beams where the rafters are inserted.
- The connection between the roof structure and the walls at the level of the longitudinal beams that are simply placed (as sleeper beams) on the top of the walls without fasteners, which does not offer the possibility of providing a joint with a higher level of safety, is yet, however, sufficiently efficient in the examined context. In fact, even if the inevitable small horizontal displacement toward the outside of the edge beam on the wall is confirmed by a horizontal crack that is visible on the outside wall of the room of the *ghorfa* at the roof-wall interface [Fig. 12(a)], it nevertheless does not seem to put the overall stability of the system at risk. In particular, it is worth highlighting that the masonry typology of the building is a mixed stone and brick masonry, bound with lime-earth mortar, that does not provide an efficient monolithic behavior due to both the hard and scarcely hewing stones and the poor mortar with a low amount of lime. Furthermore, as reported by Rovero and Fratini (2013), the average values of the mechanical properties of this masonry are rather low: compressive

strength 2.9 N/mm², Young modulus 1,340 N/mm², shear stress approximately 0.05 N/mm².

This type of damage, in fact, has been acknowledged and highlighted by the most recent Moroccan regulations, *Reglement parasismique des constructions en terre* (RPCT 2011) [(Fig. 12(b)]. The same regulations suggest, in fact, some reinforcement interventions aimed precisely at improving the connection between the edge beam of the Andalusian-type roof and the walls of the room (Fig. 13).

Seismic Vulnerability Assessment

The state of unstable equilibrium based on friction, discussed previously, which was ascertained under the assumption of only gravitational loads and the value of 0.7 for the static coefficient of friction, could also be overestimated, and the structure could be, instead, in a condition of higher risk because of the arbitrariness with which this coefficient can be assumed. In fact, according to the technical literature, reference to the dynamic coefficient of friction of 0.25 at the timber-masonry interface, which is less than half of that considered, should be made in the case of an earthquake (Rizvi 2005). This reduced value is due to the seismic actions that provoke the relevant vibrations of the structure. This is the main reason why the Moroccan-type Andalusian timber roof, as-is, cannot be considered safe with respect to possible earthquakes.

Therefore, to assess the seismic vulnerability level of the Andalusian roof, reference was made to a structural model where all the rafters were considered as perfectly pinned at the base, therefore assuming a theoretical condition of poststrengthening so as to prevent any displacement, in accordance with the RPCT (2011) recommendations. Under this assumption, reference to the dynamic coefficient of friction is omitted in this article because the sliding failure is considered prevented by fasteners.

For this reason, an additional analysis was carried out with the creation of a 3D model (FEM_4) to assess the response of the roof structure when subjected to a seismic action (Pugi and Galassi 2013). This model included both the load-bearing elements of the structural system, using monodimensional elements of the beam type and the wooden deck that supports the covering of the roof through plane plate-type elements.

Table 4. Parametric analysis: Wall thrust and collar beam axial load as a function of the inelastic displacement of the supports

Inelastic displacement (cm)	Wall thrust (kN)	Collar beam axial load (kN)
0	0.59	-0.75
0.10	0.56	-0.62
0.20	0.53	-0.49
0.30	0.51	-0.37
0.40	0.48	-0.24
0.50	0.45	-0.11
0.58	0.43	-0.0062

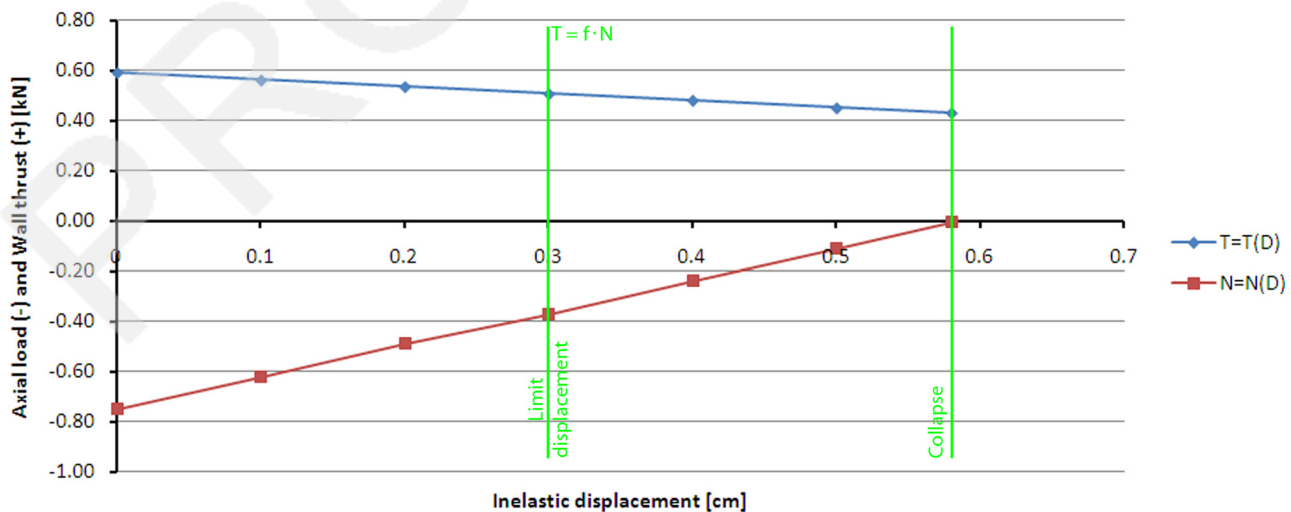
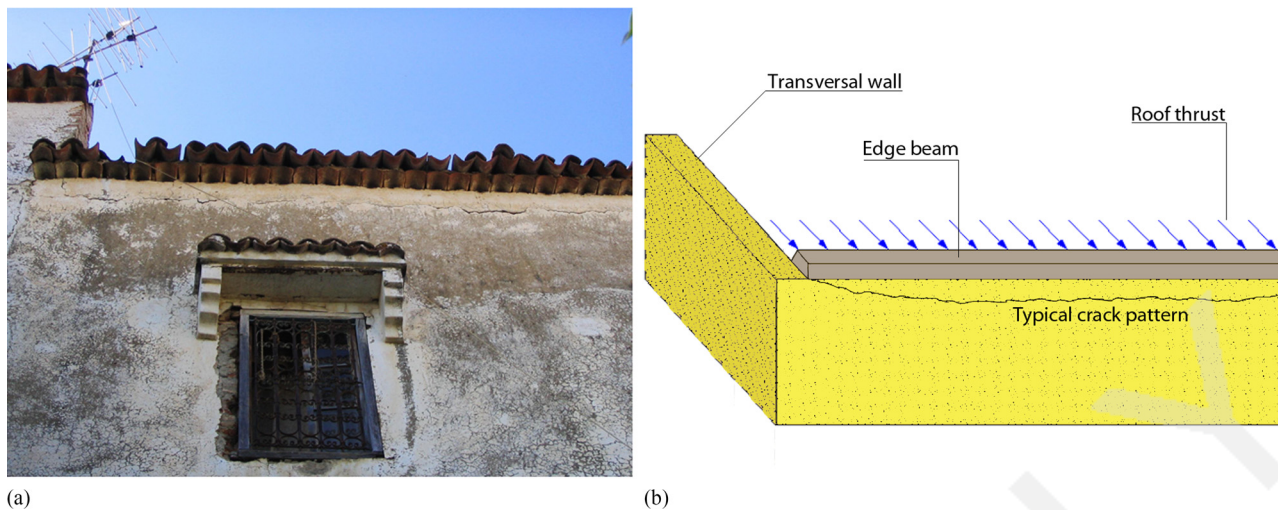


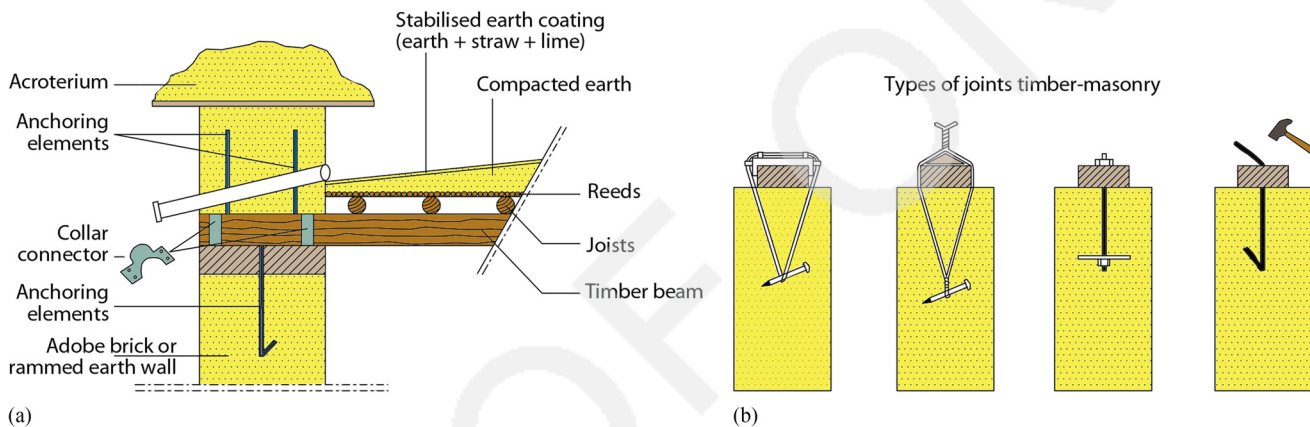
Fig. 11. Results of the parametric analysis: wall thrust T and collar beam axial load N as a function of the inelastic displacement D of the supports.

F11 : 1



(a) (b)

F12 : 1 **Fig. 12.** Horizontal crack on the wall due to the horizontal displacement of the roof's support beam: (a) *ghorfa* of Raissouni *dar* (image by Letizia
F12 : 2 Dipasquale); (b) crack scheme (adapted from RPCT 2011).



(a) (b)

F13 : 1 **Fig. 13.** (a) Reinforcement intervention for improving the connection between the edge beam of the roof and the wall of the room; (b) types of joints
F13 : 2 of the edge beam to the wall. (Adapted from RPCT 2011.)

499 Two load conditions, in addition to the one due to gravitational
500 loads, were formulated: seismic action in the transversal direction
501 (*X*-direction) and seismic action in the longitudinal direction
502 (*Z*-direction).

503 To compute the seismic action, reference was made to the
504 Moroccan *Règlement de Construction Parasismique RPS 2000—*
505 *Version 2011 (RPS 2000)*. According to these regulations, the
506 national territory of Morocco is divided into five homogeneous seis-
507 micity zones (from 0 to 4) that present approximately the same level
508 of seismic risk, with a probability equal to 10% in 50 years for the
509 recurrence of a seismic event.

510 The probability of 10% in 50 years was adopted by the regula-
511 tions envisaging a seismic event of medium intensity that can occur
512 several times during the life span of a structure. Fig. 14 presents the
513 map of the seismic areas of Morocco and shows the location of
514 Chefchaouen.

515 The city of Chefchaouen is situated in homogeneous Zone 3,
516 which is characterized by an expected seismic acceleration equal to
517 0.18*g*.

518 The combined effect of gravitational loads with the horizontal
519 seismic action evaluated as an equivalent static force, in perfect ac-
520 cordance with the provisions of the Moroccan regulations, was

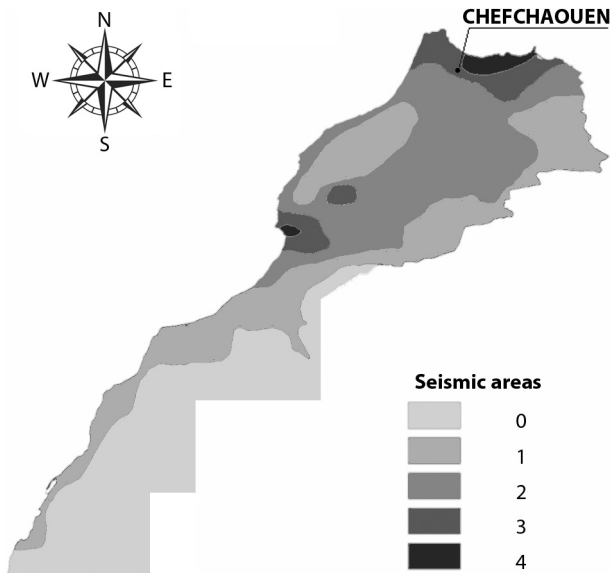
521 considered. In particular, the seismic action was inserted into the 521
522 model by applying an additional load condition with a horizontal 522
523 acceleration equal to $0.18g = 176.58 \text{ cm/sec}^2$.

524 The analysis of this fourth model (Fig. 15) clearly highlights that 524
525 the seismic action does not significantly increase the thrust trans- 525
526 mitted to the wall, nor does the axial load on the collar beam. 526
527 Therefore, considering an earthquake of medium intensity but with 527
528 a high probability of occurrence, the Andalusian-type roof shows a 528
529 good level of safety with respect to the possibility of seismic events. 529

530 It is worth highlighting that the axial compression load on the 530
531 collar beam obtained in the 3D model (-0.36 kN) is approximately 531
532 half of that obtained in the 2D model. This is due to the presence of 532
533 plate elements, inserted to simulate the deck of boards placed as loz- 533
534 enges studded to the extrados of the elements of the roof, which evi- 534
535 dently increases the stiffness of the overall structure. 535

536 Furthermore, the stress state is very low and therefore is not 536
537 capable of putting the structure at risk [Fig. 15(b)]. 537

538 The longitudinal effect of the seismic action has an even lesser 538
539 influence because the structure presents a great longitudinal stiffness 539
540 due to the dense repetitiveness of the timber elements that constitute 540
541 the structural system of the roof, made even more efficient by the re- 541
542 ciprocical connection carried out by the continuous deck of boards. 542



F14 : 1 **Fig. 14.** Map of the homogeneous seismicity zones of Morocco as a
 F14 : 2 function of the peak accelerations. (Adapted from RPS 2000.)

543 The results of the analysis for gravitational loads and seismic
 544 action are presented in Table 5.

545 Lastly, it is necessary to note that, in the authors' opinion, the
 546 strengthening interventions proposed by the Moroccan regulations
 547 consisting of the use of fasteners to anchor the timber roof (i.e., the
 548 edge beams) to the walls is effective only in the case of good-
 549 quality masonry, capable of supporting both the horizontal and ver-
 550 tical thrust provided by the roof. However, in traditional buildings
 551 of the medina, such as in the case of Raissouni *dar*, the walls are of-
 552 ten made of irregular blocks laid down without shaping due to the
 553 hardness and assembled with clay mortar. For this reason, despite
 554 the strength of the stone (which is relatively high), the overall
 555 strength of the masonry is not high because it is not guaranteed a
 556 structure capable of stress uniformity or of monolithic behavior
 557 (Rovero and Fratini 2013). Therefore, because the seismic vulner-
 558 ability of the roof is a function of the anchorage of the roof itself to
 559 the walls, which can improve the seismic response of the whole
 560 building, the authors are convinced that the proposal of the building
 561 regulations is not quite adequate and that, instead, the better way to
 562 provide anchorage of the roof structure to the wall could be to link
 563 the edge beams to the transversal beams of the attic. In this way, the
 564 horizontal thrust, which, regardless of seismic action, is yet pro-
 565 voked by the gravitational loads due to the heavy screed over the
 566 deck planking, can be nullified, and the shear failure or the over-
 567 turning of the walls can be prevented.

568 Conclusions

569 This article presents an in-depth analysis of the structural system of
 570 the Andalusian-type roof for the courtyard house, a typical building
 571 typology in northern Morocco. The analyses have been performed
 572 using numerical simulations with both 2D and 3D models carried
 573 out with the finite-element software *Straus7*.

574 In particular, the role that each structural element plays within
 575 the roof system to ensure its equilibrium was highlighted using 2D
 576 models. At the same time, it was possible to ascertain that the build-
 577 ing system presents some inherent vulnerabilities due to the

particular building technique adopted, which relies on unilateral-
 type joints and elements and on frictional supports.

580 These vulnerabilities, however, have proven to be not significant
 581 if the Andalusian structure is constructed in the context of the
 582 Moroccan territory because the climate conditions do not make it
 583 probable to have important increases in terms of load due to snow.

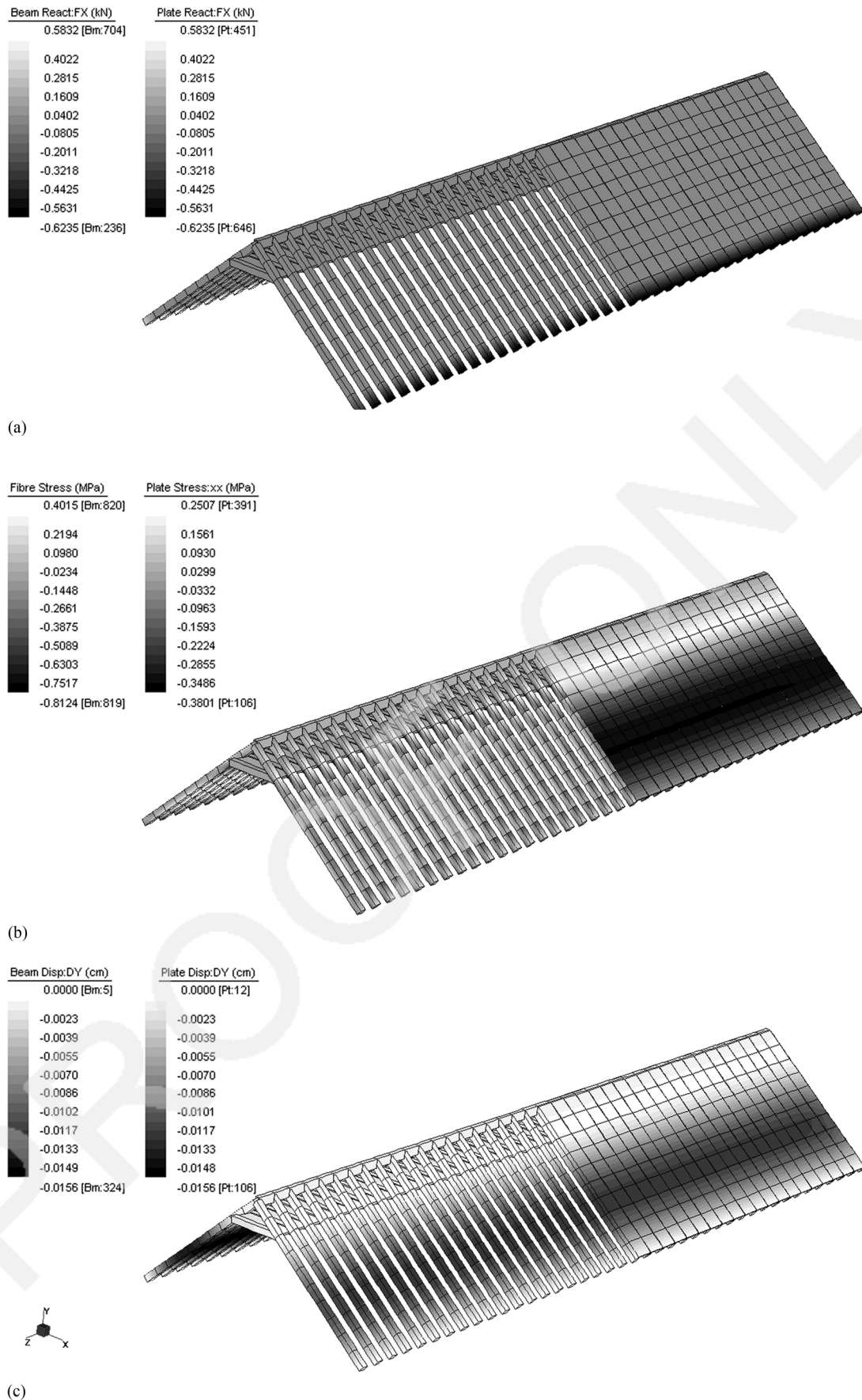
584 However, the simplicity and the typology of the connection
 585 joints among the elements of the roof structure provoke a thrust on
 586 the perimeter masonry walls that cannot be prevented. The thrust
 587 cannot be entirely balanced by the support reaction exerted in corre-
 588 spondence to the interface between the edge beam and the masonry
 589 wall, which is only based on the friction force. Therefore, a slight
 590 horizontal sliding toward the outside inevitably occurs. The visi-
 591 ble horizontal damage on the external wall at the level of the con-
 592 nection between wall and roof clearly shows the aforementioned
 593 phenomenon.

594 Local building regulations codified this type of crack as a recur-
 595 ring type of damage in Moroccan buildings where the Andalusian-
 596 type roof is used and indicate possible and specific techniques for
 597 reinforcing and improving the joint. Taking inspiration from the
 598 recommendations of the local building regulations, which propose
 599 devices for the anchorage of the roof to the walls, the seismic analy-
 600 sis was performed considering pinned supports that cannot move in
 601 a 3D model. It has been demonstrated that the effects of a seismic
 602 action, with levels of intensity predicted by the regulations, are not
 603 capable of modifying, in any significant way, the equilibrium and
 604 stability of the structure.

605 However, because the authors do not share the anchorage device
 606 proposed by the Moroccan regulations, to prevent the thrust of the
 607 roof on the masonry walls and reduce the seismic vulnerability, a
 608 strengthening intervention based on an effective connection
 609 between the transversal and the edge beams was proposed. Indeed,
 610 as mentioned previously, the actual seismic vulnerability depends
 611 on the anchorage of the roof to the walls, which is affected by the
 612 geometric and mechanical characteristics of masonry, which, in the
 613 specific case, has proven not to provide, in any way, a monolithic
 614 behavior or a high strength.

615 Finally, a parametric analysis was carried out to compute the
 616 limit value of the horizontal displacement that can turn the struc-
 617 ture into a mechanism. The analysis highlighted the fact that the
 618 limit value is never actually reached in the case of gravitational
 619 loads. In fact, the horizontal displacement, once it has begun,
 620 stops when the thrust on the edge wall decreases and is balanced
 621 by the timber–masonry friction force of supports, as a conse-
 622 quence of the new configuration of the structure due to the dis-
 623 placement itself.

624 In this regard, it is worth noting that this article is a first contribu-
 625 tion to the knowledge of the structural behavior of the Moroccan
 626 Andalusian timber roof. A fourth-step analysis based on an addi-
 627 tional numerical model that also takes into account the masonry
 628 walls could be a very realistic analytical simulation to be performed
 629 to provide a more in-depth understanding of the behavior of each
 630 timber element of the roof structure. But, as might be expected, in
 631 this model, the behavior of the collar beam (i.e., if subject to com-
 632 pression or tensile axial load) would be the consequence of the
 633 deformability of the wall rather than the rigid sliding of the edge
 634 beam on the walls, whereas, instead, the horizontal crack detected
 635 on the wall exactly under the edge beam of the roof has clearly pro-
 636 ven a rigid-cracking behavior of masonry due to the sliding failure.
 637 Because such a model would need a more accurate assessment of
 638 the mechanical properties of both timber and masonry, in addition
 639 to general knowledge regarding the geometric features of specific
 640 analyzed buildings, this issue will be addressed in a further study.



F15 : 1 **Fig. 15.** FEM_4 Model—3D model of the Andalusian-type roof for seismic analysis. Results regarding the gravitational load combination with the
 F15 : 2 seismic action in the transverse direction: (a) thrust transmitted to the walls; (b) stress state; (c) vertical displacements.

Table 5. Summary of the results of the analysis of the Andalusian-type roof for gravitational loads and seismic action

Thrust transmitted to the wall versus collar beam axial load	Load Combination 1 (gravitational loads)		Load combination 2 (gravitational loads + Earthquake X)		Load combination 3 (gravitational loads + Earthquake Z)	
	Edge frame (kN)	Central frame (kN)	Edge frame (kN)	Central frame (kN)	Edge frame (kN)	Central frame (kN)
In plane thrust (X) ^a	0.34	0.6	0.36	0.62	0.37	0.6
Vertical thrust (Y) ^b	0.44	0.78	0.46	0.79	0.47	0.78
Out-of-plane thrust (Z) ^c	0.0047	0	0.0042	0	0.013	0.023
Collar beam axial load	-0.3	-0.37	-0.3	-0.36	-0.31	-0.37

Note: Medium-intensity earthquake as defined by RPS (2000).

^aRoom transversal direction.

^bGravitational load direction.

^cRoom longitudinal direction.

641 For the aforementioned reasons, the authors will use their specific
 642 software *BrickWORK* (Galassi and Paradiso 2014; Galassi and
 643 Tempesta 2018), already used to perform the analysis of masonry
 644 constructions in earlier works (Paradiso et al. 2013, 2014a, b) and
 645 suitably developed to model the walls by rigid blocks, even
 646 assembled with heart-based mortar joints that are characterized by
 647 an elastic behavior under compressive forces and a rigid-cracking
 648 behavior under tensile forces, in agreement with the effective per-
 649 formance of the masonry that has proven not to provide a mono-
 650 lithic behavior. The results will also be compared to those provided
 651 by the use of *Straus7*, herein used to perform the analyses. It is
 652 expected to realistically describe the effect of the spreading roof on
 653 the side masonry walls and, therefore, the overall behavior of the
 654 roof-wall structure.


655 **References**

656 Anderson, G. D., and M. Rosser-Owen. 2007. *Revisiting al-Andalus: Perspectives on the material culture of Islamic Iberia and beyond*.
 657 Leiden, Netherlands: Brill.
 658 Blau, P. J. 1996. *Friction science and technology*. Boca Raton, FL: CRC.
 659 Candelas Gutiérrez, A. L. 2003. "On the origin of some 'whiteness' carpen-
 660 try rules." In *Proc., 1st Int. Congress on Construction History*, edited by
 661 S. Huerta, 505–512. Madrid: Instituto Juan de Herrera.
 662 CEN (European Committee for Standardization). 2002. *Actions on struc-
 663 tures, part 1.1*. Eurocode 1. Brussels, Belgium: CEN.
 664 Copani, P. 2006. "Ancient wooden roofs in Sicily: The heritage of Taormina."
 665 In *Proc., 2nd Int. Congress on Construction History*, 817–835. Exeter,
 666 UK: Short Run.
 667 Cruz, H., D. Yeomans, E. Tsakanika, N. Macchioni, A. Jorissen, M.
 668 Touza, M. Mannucci, and P. B. Lourenço. 2015. "Guidelines for on-
 669 site assessment of historic timber structures." *Int. J. Archit. Heritage*
 670 9 (3): 277–289. <https://doi.org/10.1080/15583058.2013.774070>.
 671 Dipasquale, L., S. Mecca, L. Rovero, and U. Tonietti. 2008. "Technical
 672 knowledge and traditional architecture in medina of Chefchaouen
 673 (Morocco)." In *Proc., 2nd Int. Conf. on Mediterranean Heritage*
 674 (*RIPAM*). Marrakech, Morocco: Université Cadi-Ayyad.
 675 Dipasquale, L., and V. Volpi. 2009. "Le coperture." In *Chefchaouen. Architettura e cultura costruttiva*, edited by S. Mecca, L. Dipasquale, L.
 676 Rovero, U. Tonietti, and V. Volpi, 167–181. Pisa, Italy: Edizioni ETS.
 677 Du Bois, A. J. 1902. *The mechanics of engineering*. New York: Wiley.
 678 Elert, G. 2017. "Friction." *The physics hypertextbook*. <https://physics.info/friction>.
 679 Galassi, S., and M. Paradiso. 2014. "BrickWORK software-aided analysis
 680 of masonry structures." *IERI Procedia*, 7: 62–70. <https://doi.org/10.1016/j.ieri.2014.08.011>.
 681 Galassi, S., M. Paradiso, and G. Tempesta. 2013. "Non-linear analysis of
 682 masonry structures subjected to external settlements." *Open J. Civ. Eng.*
 683 3 (2A): 18–26. <https://doi.org/10.4236/ojce.2013.32A003>.
 684 Galassi, S., and G. Tempesta. 2018. "Analysis of masonry block structures
 685 with unilateral frictional joints." In *Proc., 2nd Int. Conf. on Materials,*

Manufacturing and Mechanical Engineering (MMME2017). Dubai, 690
 United Arab Emirates: Emirates Research Publishing. 691
 Gorst, N. J. S., S. J. Williamson, P. F. Pallett, and L. A. Clark. 2003. 692
Friction in temporary works. Research Rep. 071 Prepared for Health 693
 and Safety Executive. Birmingham, UK: Univ. of Birmingham, School 694
 of Engineering. 695
 Grigoriev, I. S., and E. Z. Meilikhov. 1997. *Handbook of physical quanti- 696
 ties*. Boca Raton, FL: CRC. 697
 Lee, S.-H., S. Harari, H. Wong, and V. Kapila. 2005. "Friction experi- 698
 ment." *SMART—Science and Mechatronics Aided Research for 699
 Teachers 2003–2005*. <http://engineering.nyu.edu/mechatronics/smart/pdf/experiments/Project3.pdf>. 700
 Mastrodicasa, S. 1948. *Dissesti statici delle strutture edilizie. Diagnosi e 701
 consolidamento*. Milano, Italy: Hoepli. 702
 Murase, Y. 1984. "Friction of wood sliding on various materials." *J. Facil. 703
 Agric. Kyushu Univ.* 28 (4): 147–160. 704
 Nuere, E. 1989. *La carpintería de armar española. Editores instituto de 705
 conservación y restauración de bienes culturales*. Madrid, Spain: 706
 Ministerio de Cultura, Instituto de Conservacion y Restauracion de 707
 Bienes Culturales. 708
 Orlando, M., L. Salvatori, P. Spinelli, and M. De Stefano. 2016. 709
 "Displacement capacity of masonry piers: parametric numerical anal- 710
 yses versus international building codes." *Bull. Earthquake Eng.* 14 711
 (8): 2259–2271. <https://doi.org/10.1007/s10518-016-9903-x>. 712
 Paradiso, M., S. Galassi, A. Borri, and D. Sinicropi. 2013. "Reticolatus: An 713
 innovative reinforcement for irregular masonry. A numeric model." In 714
Structures and architecture: Concepts, applications and challenges, 715
 edited by P. J. S. Cruz. Boca Raton, FL: CRC. 716
 Paradiso, M., S. Galassi, and D. Sinicropi. 2014a. "Il caratteristico Forte di 717
 San Fernando de Bocachica, Colombia." In *Proc., Congress on 718
 Construction Pathology, Rehabilitation Technology and Heritage 719
 Management (Rehabend 2014)*, edited by L. Villegas, I. Iombillo, C. 720
 Liano, H. Blanco. Santander, Spain: Gráficas Iguña. 721
 Paradiso, M., S. Galassi, and D. Sinicropi. 2014b. "Stabilità di archi 722
 e volte in muratura: Metodi tradizionali e calcolo automatico." 723
 In *Proc., Congress on Construction Pathology, Rehabilitation 724
 Technology and Heritage Management (Rehabend 2014)*, edited 725
 by L. Villegas, I. Iombillo, C. Liano, H. Blanco. Santander, Spain: 726
 Gráficas Iguña. 727
 Parisi, M. A., and M. Piazza. 2000. "Mechanics of plain and retrofitted tradi- 728
 tional timber connections." *J. Struct. Eng.* 126 (12): 1395–1403. [https://doi.org/10.1061/\(ASCE\)0733-9445\(2000\)126:12\(1395\)](https://doi.org/10.1061/(ASCE)0733-9445(2000)126:12(1395)). 729
 Parisi, M. A., and C. Chesi. 2014. "Seismic vulnerability of traditional 730
 buildings: The effect of roof-masonry walls interaction." In *Proc., 10th 731
 US National Conf. on Earthquake Engineering*, Oakland, CA: 732
 Earthquake Engineering Research Institute. 733
 Parisi, M. A., M. Piazza, and C. Chesi. 2011. "Seismic response of tradi- 734
 tional timber elements and roof structures: Learning from the L'Aquila 735
 earthquake." In *Proc., 11th Int. Conf. Structural Health Assessment of 736
 Timber Structures*, 1–9. Lisbon, Portugal: Laboratorio Nacional de 737
 Engenharia Civil. 738
 Pugi, F., and S. Galassi. 2013. "Seismic analysis of masonry voussoir arches 739
 according to the Italian building code." *Ing. Sism.* 3: 33–35. 740
 741
 742

- 743 Rizvi, M. A. 2005. *Effective technical communication*. New Delhi, India: 759
744 Tata McGraw-Hill Education. 760
- 745 Rovero, L., and F. Fratini. 2013. "The medina of Chefchaouen (Morocco): 761
746 A survey on morphological and mechanical features of the masonries." 762
747 *Constr. Build. Mater.* 47: 465–479. [https://doi.org/10.1016/j.conbuildmat](https://doi.org/10.1016/j.conbuildmat.2013.05.025) 763
748 [.2013.05.025](https://doi.org/10.1016/j.conbuildmat.2013.05.025). 764
- 749 RPCT (Royaume du Maroc, Ministère de l'Interieur, Ministère de 765
750 l'Equipement et du Transport, Ministère de l'Habitat, de l'Urbanisme et 766
751 de la Politique de la Ville). 2011. *Règlement parasismique des construc-* 767
752 *tions en terre*. RPCTerre 2011. Rabat, Morocco: RPCT. 768
- 753 RPS (Royaume du Maroc, Ministère de l'Habitat et de la Politique de la 769
754 Ville). 2000. *Règlement de Construction Parasismique*. RPS 2000— 770
755 Version 2011. Decreto n. 2-02-177 du 9 hija 1422 (22 febbraio 2002). 771
756 Djibouti, Africa: RPS. 772
- 757 Ruggieri, N., S. Galassi, and G. Tempesta. 2018. "Pompeii's Stabian baths. 773
758 Mechanical behaviour assessment of selected masonry structures during 774
the 1st century seismic events." *Int. J. Archit. Heritage*. [https://doi.org](https://doi.org/10.1080/15583058.2017.1422571) 759
[/10.1080/15583058.2017.1422571](https://doi.org/10.1080/15583058.2017.1422571). 760
- Tampone, G. 2001. "Acquaintance of ancient timber structures." In *Proc., 3rd* 761
Seminar on Structural Analysis of Historic Constructions, edited by P. B. 762
Lourenço and P. Roca, 117–143. Guimaraes, Portugal: Univ. of Minho. 763
- Tampone, G. 2005. "Il complesso lignario della Cattedrale di Nicosia." 764
In *Proc., Int. Conf. on Conservation of Historic Wooden Structures*, 765
144–154. Florence, Italy: Alinea. 766
- Tampone, G., and N. Ruggieri. 2016. "State-of-the-art technology on con- 767
servation of ancient roofs with timber structure." *J. Cult. Heritage*, 22: 768
1019–1027. <https://doi.org/10.1016/j.culher.2016.05.011>. 769
- UNI (Ente Nazionale Italiano di Unificazione). 2009. *Structural timber.* 770
Strength classes. EN 338:2009. Brussels, Belgium: UNI. 771
- UNI (Ente Nazionale Italiano di Unificazione). 2010. *Structural timber.* 772
Visual strength grading. Part 3: Timber Use Fiume and Uso Trieste. 773
UNI 11035-3:2010. Milano, Italy: UNI. 774

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