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Damped Interconnection-Based Mitigation of Seismic Pounding between a R/C Tower and a Masonry Church

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Abstract. Damped interconnection represents an advanced strategy to mitigate the effects of seismic pounding between adjacent structures built at poor mutual distance. The effects of pounding can be particularly severe in slender R/C structures, including civic or bell towers. An emblematic case study falling in this class of structures, i.e. a monumental R/C bell tower constructed in the early 1960s in Florence, is analyzed in this paper. In order to assess the effects of pounding, a non-linear dynamic finite element enquiry was carried out by simulating collisions between the tower and the adjacent masonry church with a multi-spring-damper viscoelastic contact model, originally implemented in this study. The survey results show that pounding affects the seismic response of the two buildings as early as an input seismic action scaled at the amplitude of the normative basic design earthquake level. A damped interconnection-based retrofit hypothesis to prevent pounding is then proposed, which consists in linking the two structures by means of a pair of fluid-viscous dissipaters. Thanks to the added damping produced by these devices, the impacts are totally annulled, bringing the structural members of the tower to safe levels.

Keywords: Added Damping, Passive Control, Seismic Pounding Mitigation, Non-linear Fluid-Viscous Dissipaters, R/C Towers, Structural Assessment, Seismic Retrofit.

PACS: 46.70.-p

INTRODUCTION

One of the greatest sources of vulnerability of structures designed before the advent of modern Technical Standards is represented by seismic pounding, which occurs when the distance between adjacent buildings is not wide enough to avoid collision during their earthquake-induced motion. The damaging effects of pounding can be particularly severe in slender R/C structures, including various types of civic or bell towers. Indeed, these buildings are characterized by high horizontal translational deformability at least in one of the two main directions in plan, and by limited structural redundancy. In this respect, the case study analyzed in this paper is emblematic, being represented by a R/C bell tower constructed in the early 1960s to replace the existing 19th century tower of the Chiesa del Sacro Cuore in Florence (Figure 1). The new tower was built at a very narrow distance from the façade of the church due to the lack of reference Technical Standards at the time, as typical of a large stock of modern heritage R/C structures [1]. As a consequence of the little width of the gap, averagely equal to 20mm along the height, the two structures appear to be remarkably pounding-prone.

MODAL ANALYSIS OF THE BELL TOWER AND THE CHURCH

A complete finite element model of the church and the bell tower was generated by the SAP2000NL commercial calculus program [2], based on the original design documentation collected through record research, as well as on supplementary laser measurements [3]. The model of the tower was constituted by a complete mesh of frame elements. The model of the church included frame elements, for the wooden roof and the R/C dome, and shell elements, for the masonry walls. A modal analysis of the two structures was initially carried out by the model, the results of which show a first mode of the tower alone, mixed translational along the direction orthogonal to the façade (y)–rotational around the vertical axis (z), with vibration period of 1.94 s and effective associated masses equal to 80% of the seismic mass of the tower along y , and 22.4% around z . The corresponding shape is plotted in the right image of Figure 1. The second mode, concerning the bell tower alone too, is mixed translational along the direction parallel to the façade (x)–rotational, with period of 1.39 s and associated masses equal to 93.1% along x , and 66.9% around z . The third mode is translational along x –rotational, with period of 0.39 s and associated masses

equal to 3.4% along x , and 2.7% around z . The fourth, and last significant, mode is translational along y –rotational, with period of 0.38 s and associated masses equal to 14.9% along y , and 3.9% around z . The church structure features all mixed translational–rotational modes too, the former of which with period of 0.48 s. The first modes that include a significant translational contribution in y direction are the fifth and sixth ones, with vibration periods of 0.21 s and 0.17 s, equal to about 1/9 and 1/11 of the first period of the tower along the same axis. This highlights very different dynamic properties of the two structures along the potential pounding direction, as expected from their structural characteristics.

FIGURE 1. Views of the bell tower and the church and first mode shape of the finite element model of the two structures.

NON-LINEAR DYNAMIC ANALYSIS OF POUNDING

A special “multi-spring-damper” viscoelastic contact model constituted by an in-series assemblage of m linear dampers and m associated springs was devised in this study to simulate the effects of pounding in finite element time-history analyses. The response of the model is based on the sequential activation and disconnection of the dampers and relevant springs, following the variation of the interpenetration depth $\delta(t)$ (with t =time variable) between the colliding structures. This way, the resulting equivalent damping coefficient of the assemblage, c_{eq} , becomes a function of $\delta(t)$, which allows reproducing in piece-wise linear form any relation between c_{eq} and $\delta(t)$ likely to be selected in the analysis. The special relation proposed in [4]:

$$c_{eq}(t) = 2\xi \sqrt{k_H \sqrt{\delta(t)} \frac{m_1 m_2}{m_1 + m_2}} \quad (1)$$

was particularly adopted in this enquiry, where m_1, m_2 =masses of the impacting structures; k_H =stiffness of the impact force transmitting spring; and ξ = impact damping ratio, defined in [4] as the following function of the coefficient of restitution r :

$$\xi = \frac{9\sqrt{5}}{2} \frac{1 - r^2}{r[9\pi - 16] + 16] \quad (2)$$

Consistently with model [4], the impact force transmitting spring is assumed to be non-linear and governed by Hertz analytical law, i.e. expressed as a n -power law of the relative displacement between the colliding members, with the n exponent fixed at 3/2 for pounding computation. The version of the multi-spring-damper model with $m=5$ components is drawn in Figure 2a. The activation of each damper, with damping coefficient c_i — where $i=1,...,5$ in this case, and $i=1,...,m$ in general — is governed by a gap (named *gap_{Ci}* in Figure 2a), to which an initial opening is assigned. As the gap closes, the damper starts to react, adding its response to the already activated dampers. Each damper is driven to its pre-impact position by the associated linear spring, with stiffness k_{di} . The contact element is completed by the impact-transmitting Hertzian spring described above, placed in parallel to the multi-spring-damper assemblage; the existing gap at rest between the two adjacent structures (named *rest-gap*); and the gap that disconnects the damper in the rebound phase (*reb-gap*). A numerical enquiry carried out by varying the number of linear dampers from 3 to 9 highlighted that the 5-damper assemblage in Figure 2a is capable of bearing the best balance between simulation capacities and computational times. Therefore, this version of the multi-spring-damper model was incorporated at the interface between the bell tower and the church for the time-history analysis of pounding, in the positions marked by the pairs of joints denoted by letters A-A' through E-E' in Figure 2b. These joints represent the potential physical impact spots situated on the four rear columns and at the top of the rear arcade beam of the tower, and the corresponding spots on the church façade.

of the time-history of relative displacements of the bell tower with respect to the church (measured again at the top C-C' position in Figure 2b), obtained from the most demanding input ground motion scaled at BDE amplitude.

FIGURE 4. Relative displacement time-history obtained from the most demanding input motion and installation detail of one of the two dampers (plan view).

The graph shows that the maximum value of the relative displacement is equal to 13.8 mm. As this value is lower than the assumed gap depth at rest, pounding does not occur. Concerning the technical installation of the dampers, it is very simple and non-invasive from an architectural viewpoint, as shown in the design drawing in Figure 4, displaying a plan view of the intervention.

Based on the results of this study, the proposed damped-interconnection strategy appears to be an effective pounding mitigation strategy also for special slender R/C structures, such as the one examined in this paper.

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