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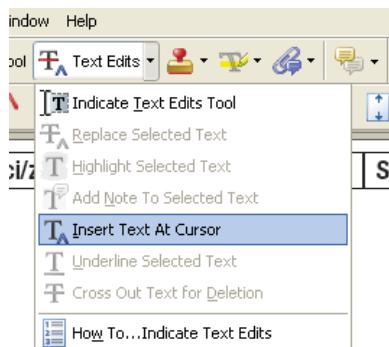
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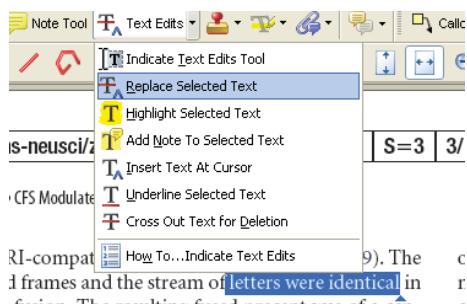
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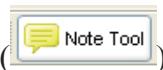
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Subject	Target contrast (%)
S1	12
S2	12
S3	15
S4	20
Mean $\pm$ SEM	14.75 $\pm$ 1.89

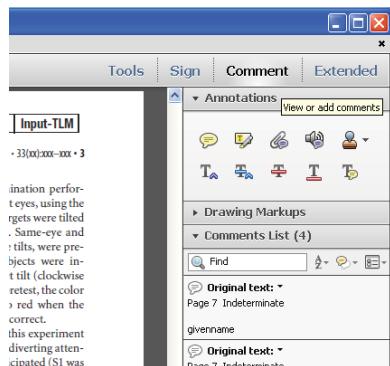
Each row corresponds to a different subject. Bottom row, mean and SEM across performance for target and mask presented to different eyes; well above chance level.

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# Andalusian Timber Roof Structure in Chefchaouen, Northern Morocco: Construction Technique and Structural Behavior

Stefano Galassi<sup>1</sup>; Letizia Dipasquale<sup>2</sup>; Nicola Ruggieri<sup>3</sup>; and Giacomo Tempesta<sup>4</sup>

**Abstract:** This article presents the results of an investigation on the building system of the Andalusian timber roof, which is widespread in northern Morocco. The structural behavior of the Andalusian timber roof structures surveyed in the medina of Chefchaouen is analyzed in depth. The analysis, carried out using finite-element models, allowed for assessment of the structural behavior of the structure but also highlighted some weaknesses that are inherent to this building system. These weaknesses are primarily due to the presence of unilateral connection elements that ensure efficiency only under specific stress conditions and also to the lack of efficiency of the connection between load-bearing elements of the roof and the surrounding walls. The detected horizontal displacement of supports explains the cracking pattern that is usually visible at the top of walls just under the level of the gutter. A parametric analysis was performed, revealing that the weaknesses of the system do not present specific criticalities in the geographic context in which the system is developed. Nevertheless, some crucial strengthening interventions are proven to be necessary for ensuring that all timber elements can suitably contribute to the overall equilibrium of the structure in the case of an earthquake. DOI: [10.1061/\(ASCE\)AE.1943-5568.0000315](https://doi.org/10.1061/(ASCE)AE.1943-5568.0000315). © 2018 American Society of Civil Engineers.

**Author keywords:** Andalusian roof; Vernacular timber roof; Timber roof–masonry interaction; Règlement de construction paralismique RPS2000; Strengthening interventions; Seismic vulnerability assessment.

## Introduction

Due to its refined and complex distinctive traits, noticeable in the geometric features of the structural elements, their organization, and the solutions adopted for the nodes, the Andalusian-type collar roof, widespread in the medina of Chefchaouen, has been the object of research, of both a historical and technological nature, and of surveys aimed at defining its construction traits (Dipasquale and Volpi 2009; Tampone 2001).

Important in-depth analysis was also carried out regarding the mechanical features of the walls of the buildings in which the Andalusian collar roof system is generally used (Rovero and Fratini 2013).

Rovero and Fratini (2013) identified three masonry types in the medina of Chefchaouen: MT1, MT2, and MT3. Type MT1 is a stone masonry made of hard limestone blocks, roughly hewn and irregular in shape. Some stone blocks running through the wall for approximately two-thirds of the thickness allow a certain transversal connection. Type MT2 is a three-headed load-bearing

brick masonry with usual block sizes of  $21 \times 10 \times 2.5$  cm or  $22 \times 10 \times 3$  cm, and the cross section of the wall is approximately 35 cm thick. Type MT3 is a mixed stone and brick masonry with a core of fine filling material and mortar in the wall section. With the aim of regularizing the wall structure and providing a connection between the internal and the external wall fabric, rows of bricks placed every 60–80 cm are generally present. In all masonry types, blocks are bound with a lime–earth mortar, executed by mixing a part of lime binder and a part of clay. The compressive strength of this mortar was evaluated to be approximately  $25 \text{ N/mm}^2$ . Lastly, walls are protected by plaster, usually of earth and lime, and painted in a thousand shades of indigo. This practice demonstrates the concern for ensuring maintenance and adequate protection of the earthy mixture against rain, without which the whole masonry system would be subjected to decay.

In this study, numerical investigations were carried out to deepen the knowledge of the overall structural behavior of the constructive typology of the Andalusian timber roof. The analyses both highlighted some inherent critical elements due to the adopted technological solutions and allowed the assessment of its vulnerabilities, not only regarding gravitational loads but also with respect to seismic actions (Parisi et al. 2011; Parisi and Chesi 2014; Ruggieri et al. 2018). The results allow for the provision of targeted solutions for conservation and safeguard.

The medina of Chefchaouen, situated in the north of Morocco, was founded by the Andalusian Arabs in 1471, who chose to build a fortified city in a strategic position to defend the region from the Portuguese invasion, not far from the source of the Ras el Maâ River. Chefchaouen had its greatest period of development in the sixteenth and seventeenth centuries as a result of the fall of the kingdom of Granada in Spain (1492), which caused the incoming of large numbers of Arab Andalusian refugees who settled in this area, attracted by the fertility of the land and its strategic position (Dipasquale et al. 2008).

It is precisely due to the influence of the Spanish Andalusian culture, and to the fortunate integration with local Berber and Islamic

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70 traditions, that the medina of Chefchaouen efficiently represents the  
 71 process of development of an architectural and urban culture with  
 72 original and particularly significant traits.

73 The most recurrent traditional roofs in the medina of Chefchaouen  
 74 are of the double-pitched type. These roofs, as can be deduced from  
 75 observation, in-field surveys, and interviews with local master  
 76 builders (*madlem*), are classifiable into two structural categories:  
 77 the Berber structure and the Andalusian structure.

78 The former (Fig. 1) is the simplest and also the oldest one. The  
 79 ridge beam is supported by a bearing structure composed of very  
 80 close sloping rafters. The pitch slope is between 30 and 40%. In the  
 81 case of wide spans to be covered (more than 5 m), the Berber system  
 82 is aided by additional elements, namely, two principal rafters joined  
 83 to a horizontal beam. The king post is directly joined to the tie-  
 84 beam, constituting a not-beneficial concentrated load for the tense  
 85 element. Hence, this configuration makes the static behavior of this  
 86 constructive typology ineffective, which indeed always shows very  
 87 deformed structural elements. The behavior of the Berber structure  
 88 is currently under analysis, and the results of the investigation will  
 89 be provided in a following paper.

90 The Andalusian structure is more complex and interesting from  
 91 the constructive point of view; it uses well-squared and often finely  
 92 decorated wooden elements (Dipasquale and Volpi 2009). The  
 93 most widely used wood species in Chefchaouen, all from local sour-  
 94 ces, are cedar (*Ærz*), fir (*soha*), and red fir (*sanawbar*). Whereas cedar  
 95 is often used for decorations, fir and red fir are mainly used to  
 96 build roof structures.

97 A key element in Moorish architecture, the Andalusian roof is a  
 98 recurrent building system in central and southern Spain (Anderson  
 99 and Rosser-Owen 2007), where it is called *armadura de par y*  
 100 *nudillo*, meaning a structure of rafters (*pares*) and collar beams  
 101 (*nudillos*). During the period of Arab domination (711–1492), this  
 102 region was known as Al-Andalus and received architectural and  
 103 artistic influence from the Muslim culture and from the North  
 104 African Berbers and the classical Roman tradition already present  
 105 in Andalusia. In this area, it is still possible to find examples of  
 106 *armadura de par y nudillo*, especially in religious buildings con-  
 107 verted into churches and in noble palaces (Nuere 1989; Candelas  
 108 Gutiérrez 2003). In the medina of Chefchaouen, the Andalusian-  
 109 type wooden structure was imported by the Andalusian master  
 110 builders who settled there and was widely used—with substantial  
 111 modifications regarding the constructive technique mostly used in  
 112 Spain, which are not noticeable at first sight—for the roofs of the  
 113 rooms of the courtyard house and the bays of mosques. A very simi-  
 114 lar structural organization is also emphasized in many church roofs  
 115 in the Sicily region (Copani 2006). An eminent example is the  
 116 Nicosia cathedral (Catania) that dates back to the fifteenth century  
 117 and derived from the Arab domination (ninth to eleventh centuries)  
 118 and consequent constructive culture influence on the Sicily region  
 119 (Tampone 2005).

120 The article is organized as follows: The constructive system of  
 121 the Andalusian timber roof structure is described in the second

122 section, and a fundamental comparison between the Moroccan and  
 123 the Spanish Andalusian version is also provided. The third section  
 124 is devoted to the reference case study of an ancient courtyard house  
 125 in the medina of Chefchaouen, in which the structure type under  
 126 analysis is found. The role of each timber element is investigated,  
 127 and the structural behavior is assessed. In the same section, the  
 128 results of the analyses, which highlight some inherent weaknesses  
 129 of this type of structure, are summarized. The results explain and  
 130 are coherent with the external cracking pattern detected. The fourth  
 131 section deals with the seismic behavior assessment of the structure,  
 132 taking into account, as a reference, the provisions in the local regu-  
 133 lations. The final section presents concluding remarks.

## Construction Analysis

134

The structure of the Andalusian-type roof is constituted by a single-  
 135 frame double-pitched roof, made with the use of coupled rafters  
 136 placed with a slope of approximately 85% and oriented in accordance  
 137 with the shorter side of the room to be covered. The span varies from  
 138 3 to 4 m, in courtyard houses, up to 7 m, in the case of mosques.

The coupled rafters are counterposed and connected at the ridge  
 140 with the use of a plank, which is particularly useful during the phase  
 141 of setting the structure with the aim of maintaining the spacing estab-  
 142 lished for placing the other elements. Therefore, the roof carpentry  
 143 work does not include an actual ridge beam. Every coupled rafter  
 144 includes a transversal connection timber element (collar beam) with  
 145 a section of 5–7 by 10 to 15 cm and a length of approximately 60 to  
 146 65 cm, whose far ends are adequately shaped and carved so as to pro-  
 147 vide suitable support for the connecting joints (Fig. 2).

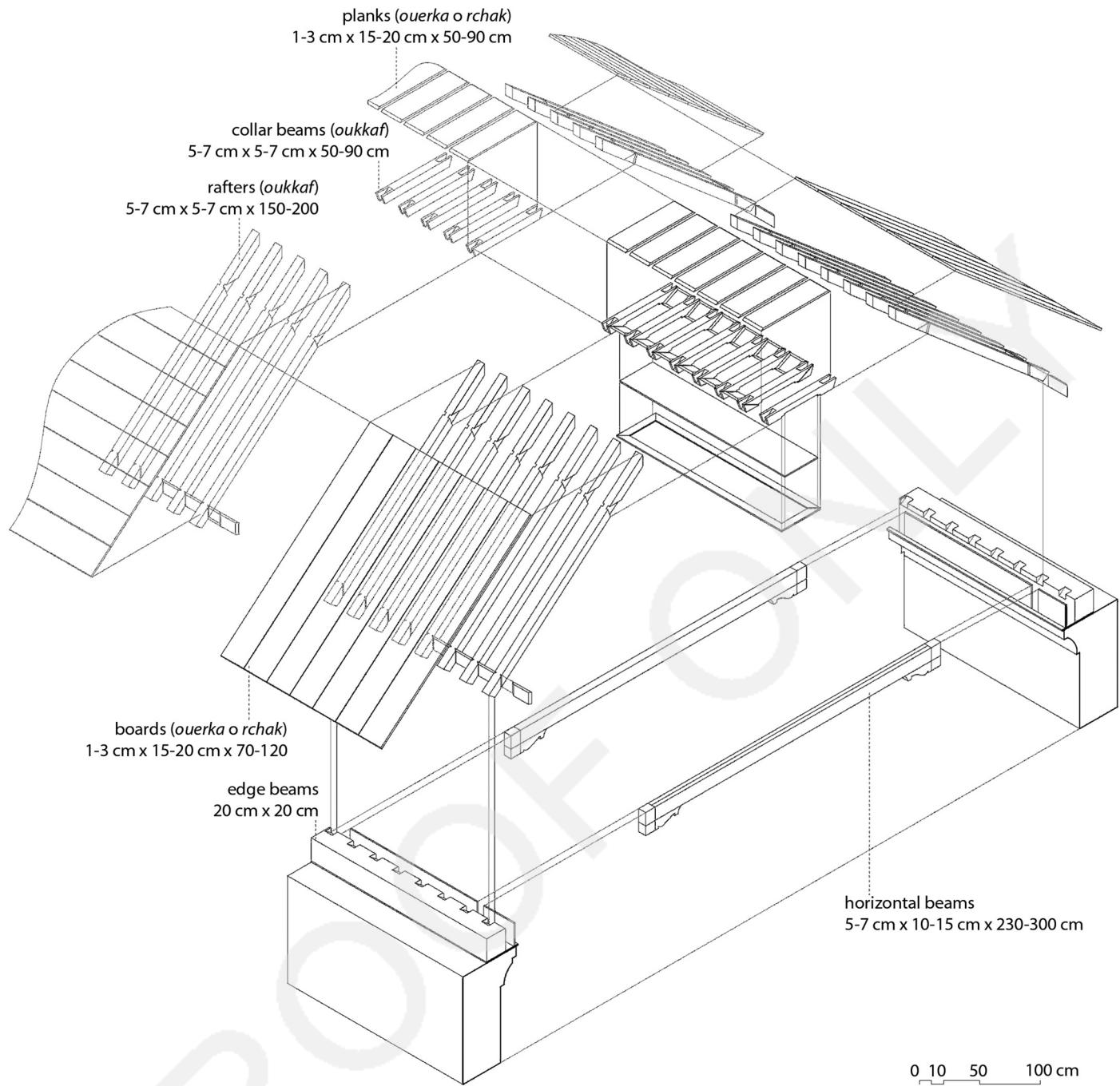
The collar beams, placed in the upper part of the structure at a  
 149 distance from the ridge of about one-fifth of the rafters' length,  
 150 support wooden planks that, with their thickness, are wedged into  
 151 the grooves carved in the rafters. In that same spot, additional  
 152 planks completing the connecting system are placed, orthogo-  
 153 nally arranged to the roof surface and wedged into the corre-  
 154 sponding grooves in the collar beams [Figs. 3(a and c)].

The central part of the roof presents an additional set of boards at  
 156 the lower edge of the collar beams, wedged to the collar beams and  
 157 nailed to the boards [Fig. 3(b)]. The ensemble of these elements  
 158 constitutes the *bsat* (Dipasquale and Volpi 2009).

From the geometric and constructive features of the joint  
 160 between the collar beam and the rafter, it can be noticed how that  
 161 device provides only a unilateral connection, capable of transferring  
 162 compression forces but inefficient if subjected to tensile forces  
 163 [Figs. 3(b and c)]. In the constructive technique of the aforemen-  
 164 tioned structural system, the boards that constitute the external deck  
 165 of the roof, which are directly nailed to the rafters, assure a good  
 166 overall stiffness and, at the same time, nullify the tendency to stack  
 167 the individual structural units, preserving the spacing between them  
 168 unchanged (Tampone and Ruggieri 2016).



Fig. 1. (a) Sectional elevation; (b) Berber roof structure (image by Letizia Dipasquale).



**Fig. 2.** Exploded view drawing of the Andalusian-style roof: Procedure of assembly of the elements.

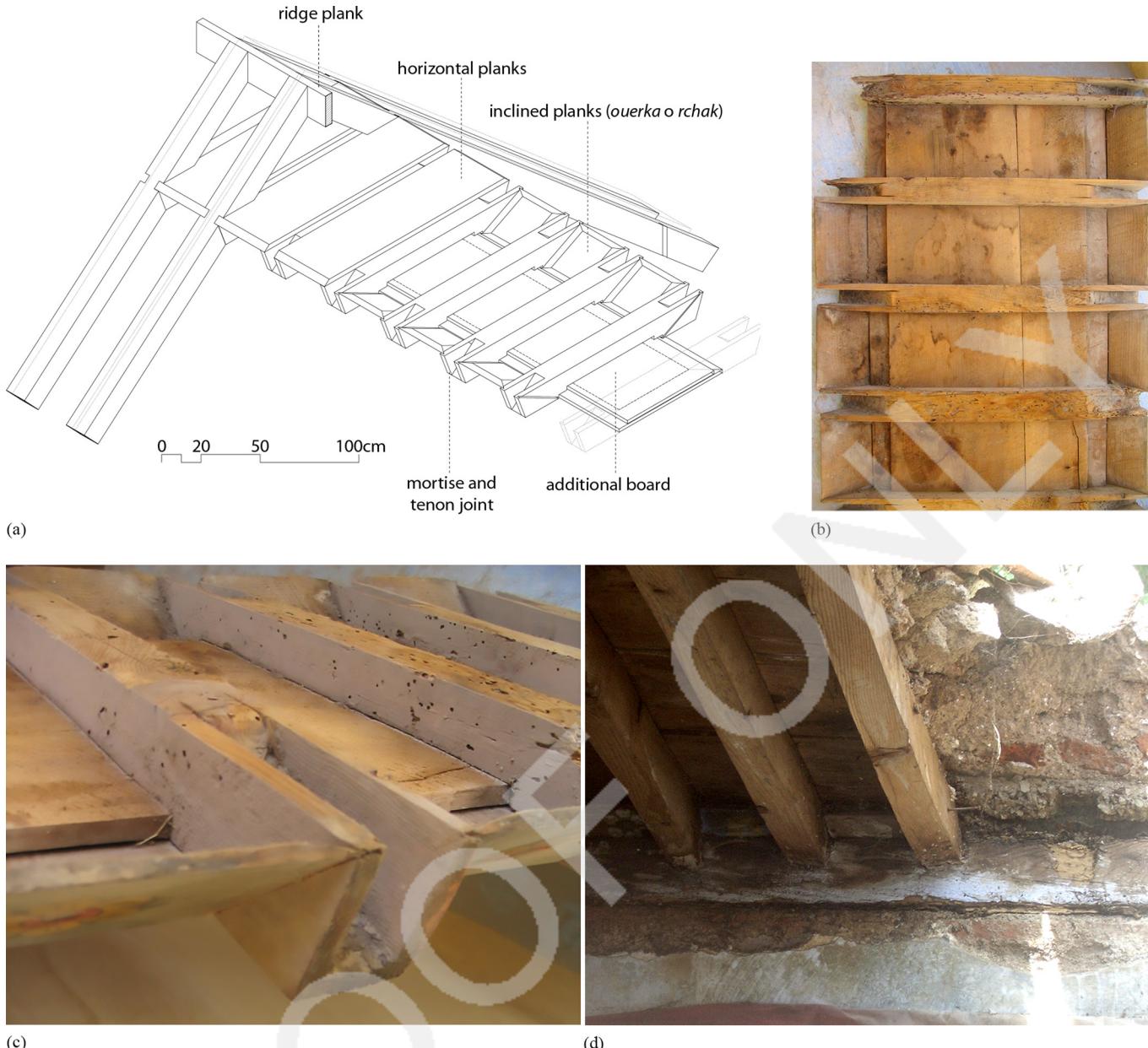
F2 : 1

170 The bases of each couple of rafters are connected to two edge  
 171 beams with the rectangular cross section, placed at the top of the  
 172 longitudinal walls, which provide support for the roof structure and  
 173 allow a good distribution of the actions transmitted on the masonry  
 174 walls. The connection between the main elements of the roof and  
 175 the edge beams is obtained through a half-lap joint, that is, a cavity  
 176 hollowed out on the upper corner of the edge beams, which holds  
 177 the head of the rafter [Fig. 3(d)]. The edge beam thus has a square  
 178 and beveled section alternately. The general carpentry elements are  
 179 presented in Table 1.

180 With the purpose of providing the roof deck for both pitches,  
 181 wooden boards are nailed to the upper edge of a couple of rafters,  
 182 arranged transversally to the room at the center and longitudinally  
 183 at the ends (Fig. 2).

184 The deck provides support for the screed and for the roofing  
 185 tiles. The screed, with a thickness between 5 and 12 cm, is constitu-  
 186 ted by a mix of earth, lime, pebbles, and fragments of bricks. The  
 187 curved tiles (which are made of a mix of clay, sand, straw, and or-  
 188 ganic elements, molded by hand and baked in traditional wood  
 189 ovens) are placed in two superposed inverted layers directly on it.  
 190 Fig. 4 shows the covering and the arrangement of the tiles to provide  
 191 two typical gutter systems, the simple and the protruding gutter.  
 192 These systems are aimed at directing the rainwater away from the  
 193 masonry walls and preserving the earth mortar and plaster.

194 Additional horizontal beams (Fig. 5) are often placed at the level  
 195 of the edge beams, with a rectangular cross section of approxi-  
 196 mately 7 by 15 cm, arranged transversally to the room without a  
 197 structural connection with the edge beams, and are thus unable to



F3 : 1 **Fig. 3.** (a) Schematic representation of the longitudinal connection of the structural system's coupled rafters-collar beam with the use of inclined and horizontal planks placed into grooves carved on the heads of the collar beam and in the tenons on the rafters; (b) set of boards at the lower edge of the F3 : 2 collar beams; (c) detail of the mortise joint on the collar beams; (d) detail of the node between rafter and edge beam. [Images (b-d) by Letizia F3 : 3 Dipasquale.]

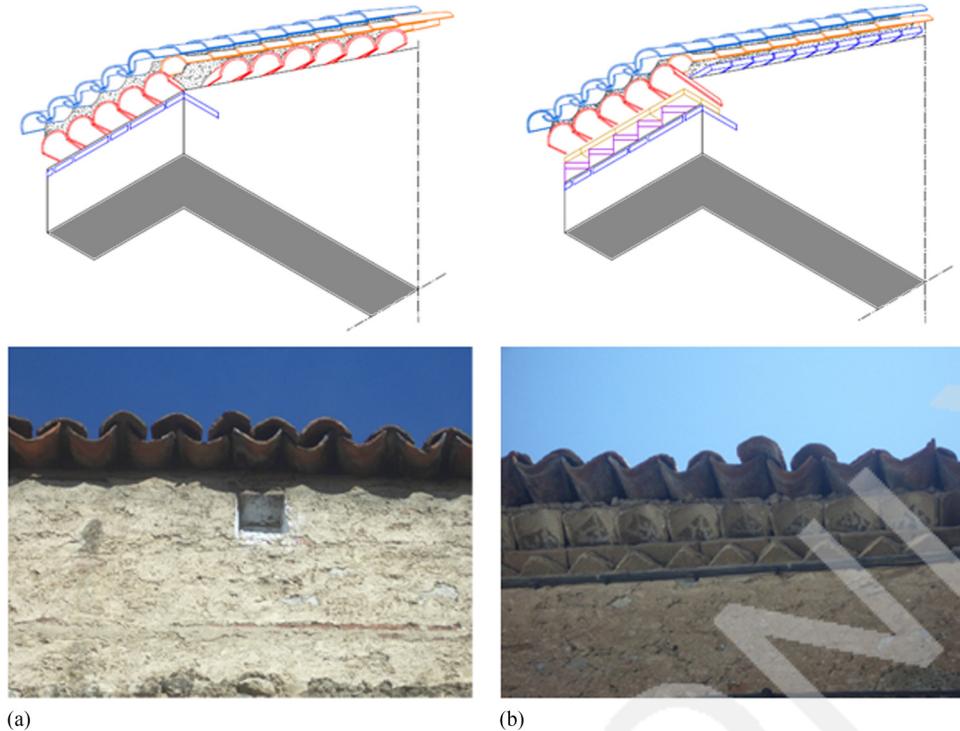
**Table 1.** Dimensions of the carpentry elements

Wooden elements	Size (cm by cm)
Rafter ( <i>Oukkaf</i> )	5-7 by 5-7
Collar beam	5-7 by 10-15
Ridge plank	4-5 by 10-12
Planks	1-3 by 15-20

provide contribution to bear the thrust from the roof structure, unlike a traditional king post roof truss or the *par y nudillo* timber roof in Spain, from which the Moroccan version was derived. In fact, in the Spanish Andalusian system (Nuere 1989), the transversal beams can behave as tie-beams, given their U-shaped grooves where the longitudinal edge beams are placed, forming a crosslap

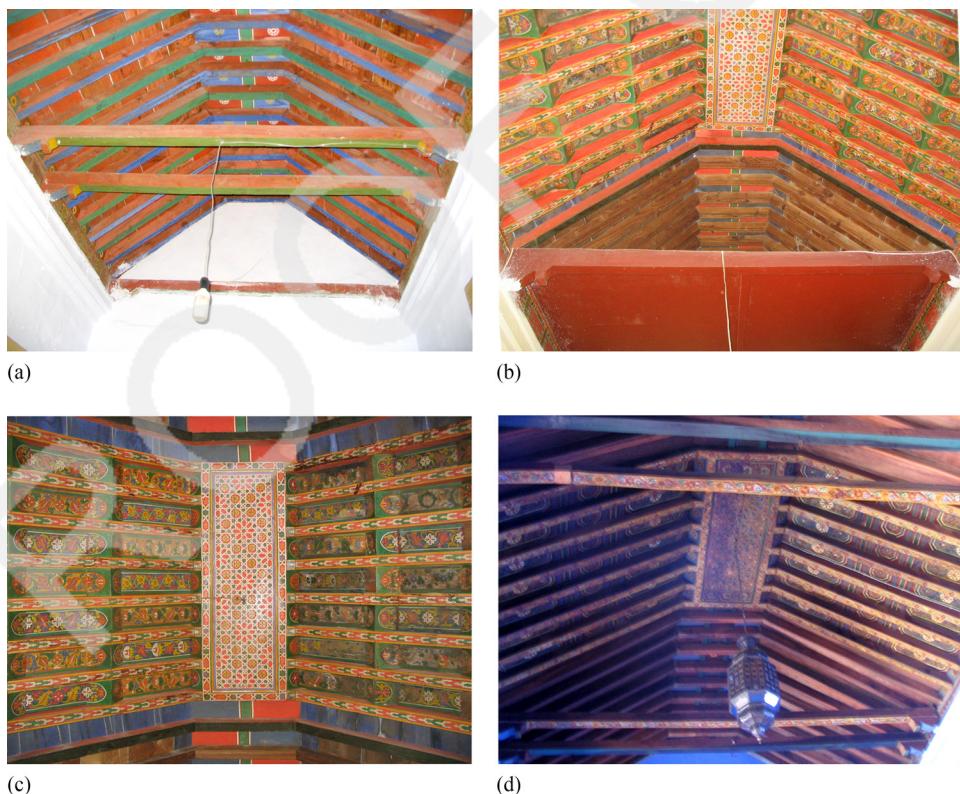
joint (compare Figs. 2 and 5 with Fig. 6). Instead, in the Moroccan version, the transversal beams do not work as actual ties because the connection with the edge beams is not sufficient to assure such a role because the link, which is not present in all cases, is made of simple metal brackets or nails with a wooden corbel. Considering this node geometry, although a pair of nails is usually present, the connection cannot transfer the tensile force from the transversal beams to the edge beams. These beams, usually placed in couples [Figs. 5(a and d)], with a variable spacing, usually alternate, of approximately 1.30 and 0.45 m, have only the function of providing support for a possible attic, usually used as a storeroom or garret [Fig. 5(b)]. With the aim of reducing the span of these beams and providing a suitable end support, the walls include a series of bricks that protrude approximately 15 cm, with the addition of the aforementioned wooden corbel.

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**Fig. 4.** Gutter systems: (a) simple gutter; (b) protruding gutter. (Images by Letizia Dipasquale.)

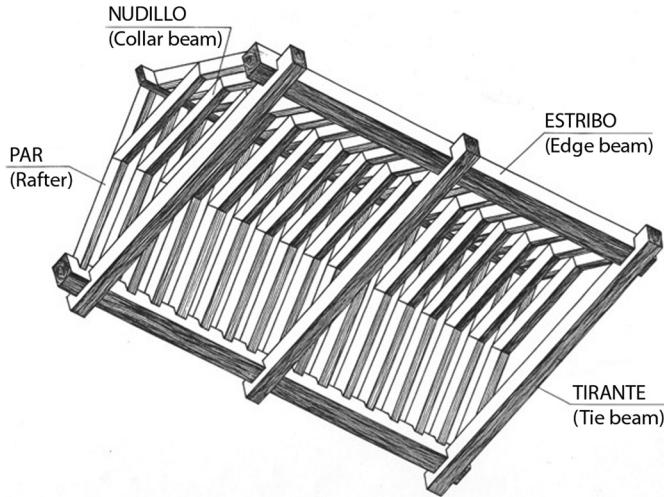
F4 : 1



**Fig. 5.** Internal view of an Andalusian-type roof with floral and arabesque decorations: (a) without mezzanine attic; (b) with mezzanine attic; (c) details of the colored decorations; (d) details of the transversal beams. (Images by Letizia Dipasquale.)

219 The system is characterized by great simplicity of execution. In  
220 fact, retracing the main construction phases, first the edge beams  
221 are placed on the side walls, then each pair of rafters is erected and

placed together with the transversal connecting element. The base 222 of the rafters is joined to the edge beams by way of the notch carved 223 into the edge beams themselves, without nails. The next pair could 224



F6 : 1 Fig. 6. Andalusian-type roof in Spain (with tie-beam elements) known as *armadura de par y nudillo*.  
F6 : 2

be placed after the insertion of both the inclined and horizontal planks, which connect them to the previous pair. The spacing between the rafters, very reduced (between 10 and 20 cm), corresponds approximately to double their width. The building procedure just described is very similar to a sort of modern prefabrication system. The site was always headed by a *maâlem* (i.e., master builder), who had an excellent knowledge of geometry and was capable of drawing and representing, by in-scale models, the structure of the roof to be built. It was the *maâlem* himself who established the dimension and spacing of the elements, based on the size of the room and the slope of the roof.

The inner surface of the Andalusian-style roof is often painted with geometric, floral, and arabesque patterns and framed by additional wooden planks [Figs. 5(b-d)].

## Case Study

With the purpose of assessing the safety of the structural system used in the configuration of the Andalusian roof and in attempt to highlight its vulnerabilities (Cruz et al. 2015), both inherent and deriving from possible seismic events, a series of numerical simulations was carried out with the finite-element method (FEM) software Straus7. The reference case study of the Raïssouni *dar* was examined, which is the oldest courtyard house in the medina of Chefchaouen. Built by the founder of the city, Moulay Ali Ben Rachid, the house underwent several transformations throughout the years, and today the building represents an example in which the constructive solutions adopted are among the most refined, technologically more advanced and structurally more correct. In particular, the roof analyzed is that of the *ghorfa* (bedroom), the common space in which the family nucleus spends most of its domestic life.

Dimensions of both masonry walls and roof timber elements were ascertained by on-site inspections during surveys. The covered room is an  $8.60 \times 2.90$  m rectangular space with 0.35-m-thick walls in stone and bricks set in a mortar of mixed lime and earth.

The load-bearing structure of the roof, arranged according to the typical Andalusian configuration, is constituted by the usual sequence of counterposed couples of rafters set with a slope of 72.5% (approximately 40°) and placed with a spacing of 0.215 m (Fig. 7).

The mechanical and dimensional characteristics of the elements that form the structure, assumed in the numerical models, are presented in Table 2 and Fig. 7. Because experimental data were not available for the mechanical features, reference was made to conventional values from the standard UNI 11035-3:2010 (UNI 2010). These values, and in particular the specific weight, coincide with those provided by Eurocode 1 (CEN 2002) relative to the timber strength class C18 reported in the standard UNI EN 338:2009 (UNI 2009), which are obtained for timber at a moisture content consistent with a temperature of 20°C and a relative humidity of 65%. Such values were assumed in this study because they are coherent with the typical Mediterranean climate of Chefchaouen, characterized by high humidity and quite high temperatures (the mean temperature is equal to approximately 17°C).

In the structural model, the transversal beams that provide support for the attic deck were considered due to the effective lack of connection with the edge beams of the roof detected.

Table 3 shows the incidence of self-weight loads acting on each rafter, listed both per unit area and as load uniformly distributed on the axis of the element.

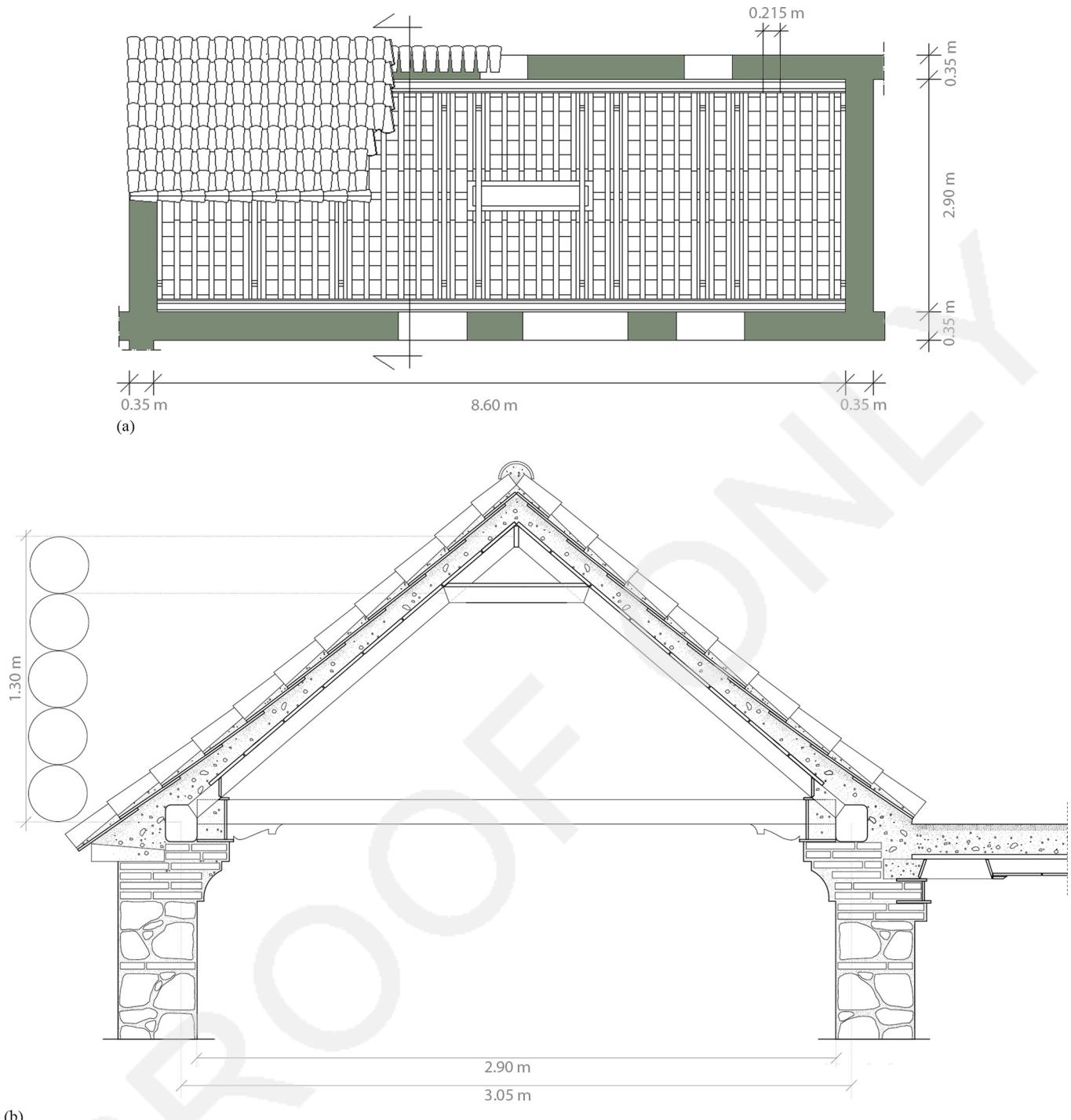
## Structural Behavior Assessment of the Timber Elements

Because the roof structure is generated by a repetitive elemental frame, composed of a couple of sloping rafters and the collar beam, a fundamental investigation to detect the exact function of each timber element was carried out by the analysis of very simple two-dimensional (2D) models. Three configurations of this model were conceived, where only the external supports were changed. All geometric and dimensional features of the elements were preserved, as were the rafter-to-rafter and rafter-to-collar beam links, assumed as internal hinges. The collar beam was considered as a truss, capable of transmitting the axial load but also subjected to the bending moment due to its self-weight.

Through direct site inspections, it was ascertained that the roof system is simply supported on the room walls, by means of the edge beams, without fasteners. Based on this observation, a first model was conceived (Model FEM\_1) as supported on a roller and a pinned support, respectively. As expected, in this model the collar beam is necessarily subjected to a tensile force, taking the role of the lacking tie-beam at the base of the system. However, it is not coherent with the technological solution adopted in the tenon and mortise joints between the collar beam and the coupled rafters (Parisi and Piazza 2000); therefore, the boundary conditions of this model cannot effectively describe the behavior of the actual structure. In Fig. 8, the results of the analysis are shown; specifically, it is worth noting the following:

1. The structure transfers to the walls an exclusively vertical action equal to 0.73 kN.
2. The tensile force in the collar beam, equal to 2.07 kN, is certainly not negligible.
3. The very high horizontal displacement of the roller support equal to 4.40 cm directed toward the exterior of the room, which is also lowered by the chain effect of the collar beam according to the assumed hypotheses of this model, actually would detach the collar beam from the couple of rafters and transform the structural system into a collapse mechanism due to the spreading of the two pitches.

Therefore, from the considerations summarized in the previous list, it can be easily deduced that the structure was originally intended by the Moroccan master builders (*maâlem*) as a statically indeterminate system whose supports on the walls work as two pinned joints. Furthermore, because the two supports of the roof to the walls are of the same type, it should not be correct to assume



**Fig. 7.** Ghorfa roof structure in the Raissouni dar: (a) plan; (b) transverse section.

F7 : 1

**Table 2.** Mechanical and dimensional characteristics assumed for the timber elements of the roof structure

FIR elements	Size (cm by cm)	Specific weight ( $\text{kN}\cdot\text{m}^3$ )	Elastic modulus ( $\text{N/mm}^2$ )	Flexural strength ( $\text{N/mm}^2$ )
Ridge plank	5 by 10	3.8	7,200	28
Rafter	6.5 by 7.2			
Collar beam	6.5 by 7.2			
Edge beam	14.5 by 16.5			

Source: Data from CEN (2002); UNI (2009, 2010).

**Table 3.** Self-weight load acting on each rafter

Roof floor	Description	Thickness (cm)	Incidence of load per unit area (kN·m <sup>-2</sup> )	Uniformly distributed load (kN·m)
Deck planking	Wooden boards	1.5	0.057	0.012
Screed	Mix of earth, lime, pebbles, and fragments of bricks	8	0.96	0.21
Covering tiles	Mix of clay, sand, straw, and organic elements, molded by hand	—	0.6	0.13

one as a hinge and the other as a roller. Accordingly, a second model (FEM\_2) was made that considers these supports (Fig. 9).

The analysis of this second model underlined that the collar beam behaves as a strut under a compression force equal to 0.75 kN, in agreement with the technological solution adopted for building the joint under study, and that the structure transfers to the walls a horizontal thrust equal to 0.59 kN and a vertical action of 0.73 kN.

Furthermore, the stress state of the structural elements was found to vary in a very reduced range, between  $-1.33$  and  $+1.22 \text{ N/mm}^2$ , values that are much lower than the limit values of the fir wood. The most stressed areas were detected at the midspan of the rafter and near the joint with the collar beam. The maximum vertical displacement, detected at the midspan of each rafter, was equal to 0.05 cm, a value that is much lower than the limit value (0.53 cm, i.e., 1/300 of the span). Therefore, in this model the strength and serviceability verifications are also satisfied.

According to the results just presented, the behavior of the second model could match the actual behavior of the real structure. However, it is necessary to note that the thrust provoked by the rafters on the masonry walls must rely only on the friction reaction that is produced in the timber–masonry interface, between the edge beam and the wall, due to the lack of the tie-beam at the base of the roof system.

A shear-sliding verification was, therefore, carried out at the timber–masonry interface. The criterion of Coulomb's friction cone was adopted, and the value of the static coefficient of friction was taken from the technical literature.

Because the value of the actual friction coefficient was not available, reference was made to values from the technical literature, which range from 0.4 to 0.7 (Du Bois 1902; Mastrodicasa 1948; Murase 1984; Blau 1996; Grigoriev and Meilikov 1997; Elert 2017; Gorst et al. 2003; Lee et al. 2005). The highest value of 0.7 was chosen as the reference value to assess if the roof structure had the propensity to slip on the walls already in the optimal condition due to the highest friction reaction.

The computed shear force (i.e., the horizontal thrust transmitted by the structure of the roof to the wall), equal to 0.59 kN, is slightly higher than the maximum value of the friction reaction that the beam–wall joint can exert ( $f \cdot N = 0.7 \times 0.73 = 0.51 \text{ kN}$ ). Therefore, this condition would highlight the propensity of the structure to suffer a horizontal displacement exactly in correspondence to the edge beam.

To deepen this phenomenon, an additional analysis was carried out in a further model (FEM\_3), simulating the presence of a horizontal inelastic displacement at the level of the edge beams.

The third model (Fig. 10) was analyzed using a step-by-step procedure of increasing inelastic displacements applied to the external supports (Galassi et al. 2013; Orlando et al. 2016). The process was interrupted at the step in which the axial load in the collar beam became positive (traction) and, therefore, incompatible with the actual performance of the element. The results of this parametric analysis are presented in Table 4. The values reported in Table 4 are also graphically represented in Fig. 11.

The diagrams presented in Fig. 11 highlight that, for a value of the inelastic displacement equal to 0.30 cm for both supports, the horizontal thrust transmitted by the structure on the edge beam is exactly equal to the friction reaction, which ensures the equilibrium

of the system. This means that, in correspondence to that value, the sliding of the edge beam toward the exterior stops, and the structure finds a new equilibrated configuration.

The last displacement considered in the parametric analysis (approximately 0.6 mm), at which the axial load in the collar beam changes sign, corresponds to the collapse of the structure, which transforms into a mechanism. The ultimate displacement detected is twice the displacement at which the thrust on the wall is balanced by the friction force. This highlights considerable safety in the case of gravitational loads.

### Discussion of Results and Strengthening Interventions

The timber roof structure of the Raissouni *dar* represents a case study that is sufficiently representative of the Andalusian-type roof in Morocco.

The numerical analyses carried out highlighted the structural behavior of the timber structure in which a significant role is played by the collar beam that connects, in proximity of the ridge of the roof, the two counterposed rafters that form the covering surface. It was ascertained that, given the peculiar tenon and mortise joint, the horizontal beam must behave as a strut and contributes, on the one hand, to containing the flexural deformation of the roof and, on the other, confers a higher degree of safety to the efficiency of the hinge-joint between the two rafters near the cusp, especially in the presence of nonsymmetrical actions.

In detail, it was proven that the Moroccan-type Andalusian roof system shows a general structural consistency and a sufficient level of safety in the case of gravitational loads, even if the structure is in a state of unstable equilibrium ensured only by the friction between the edge beam and the masonry.

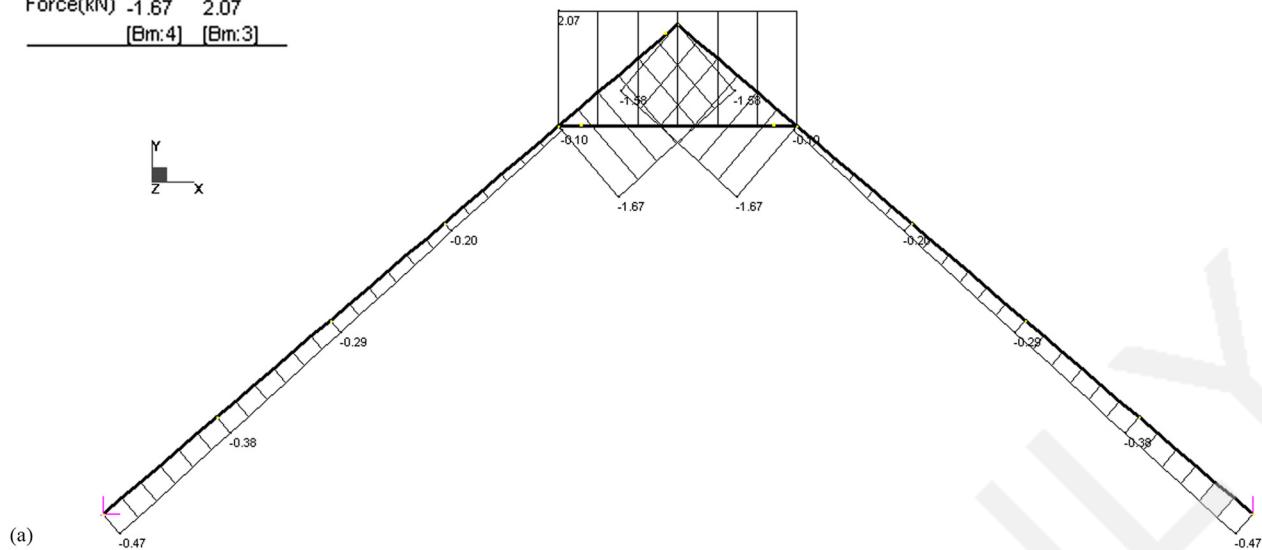
The main reasons can be listed as follows:

- The spacing between the rafters is moderate (21.5 cm).
- The high slope of the roof pitch and, therefore, of the elements that constitute the load-bearing structure (over 70%), together with the low incidence of the dead loads as a result of the moderate spacing between the rafters, determines a very low horizontal thrust on the wall.
- Given its location in Morocco, variable loads cannot reach significant values; snow, for example, is not a possible load condition. Thus, any increase over time in the stress on the load-bearing timber elements and of the thrust on the walls is not possible.

However, negative characteristics of the roof affecting the safety for gravitational loads and meaningful critical elements are identified as follows:

- The approximately 8- to 10-cm-thick screed over the planks is an extremely heavy load, but its distribution on each element does not reach very high values thanks to the reduced spacing.
- The tenon and mortise joint between the collar beam and the timber elements of the pitch acts as a unilateral connection.
- There is a lack of an actual ridge beam.
- The edge beams are simply supported on the masonry walls without fasteners.

	MIN	MAX
Force(kN)	-1.67	2.07
	[Bm:4]	[Bm:3]

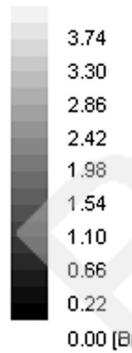


Fibre Stress (MPa)  
9.52 [Bm:9]



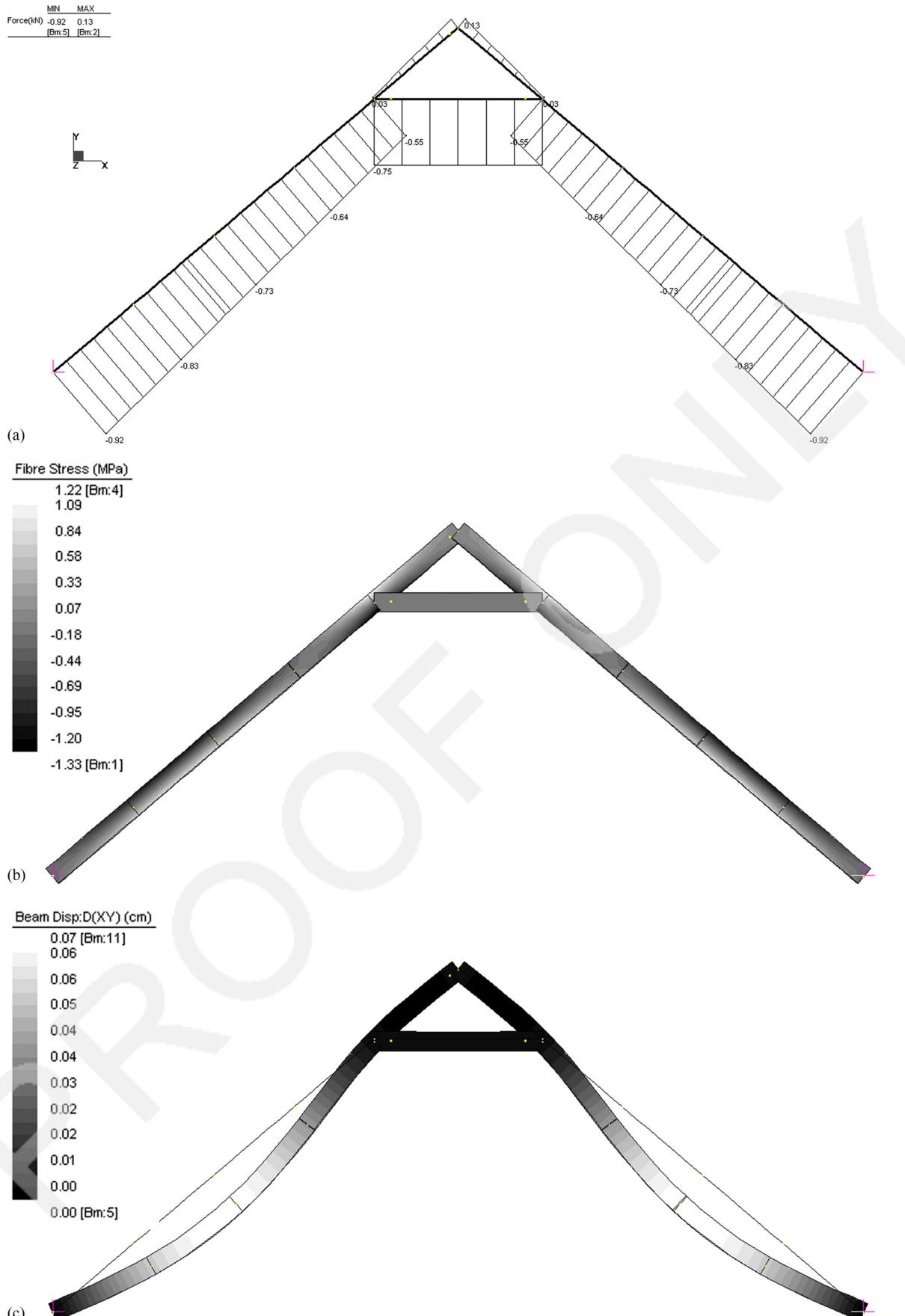
(b)

Beam Disp:D(XY) (cm)  
4.40 [Bm:5]

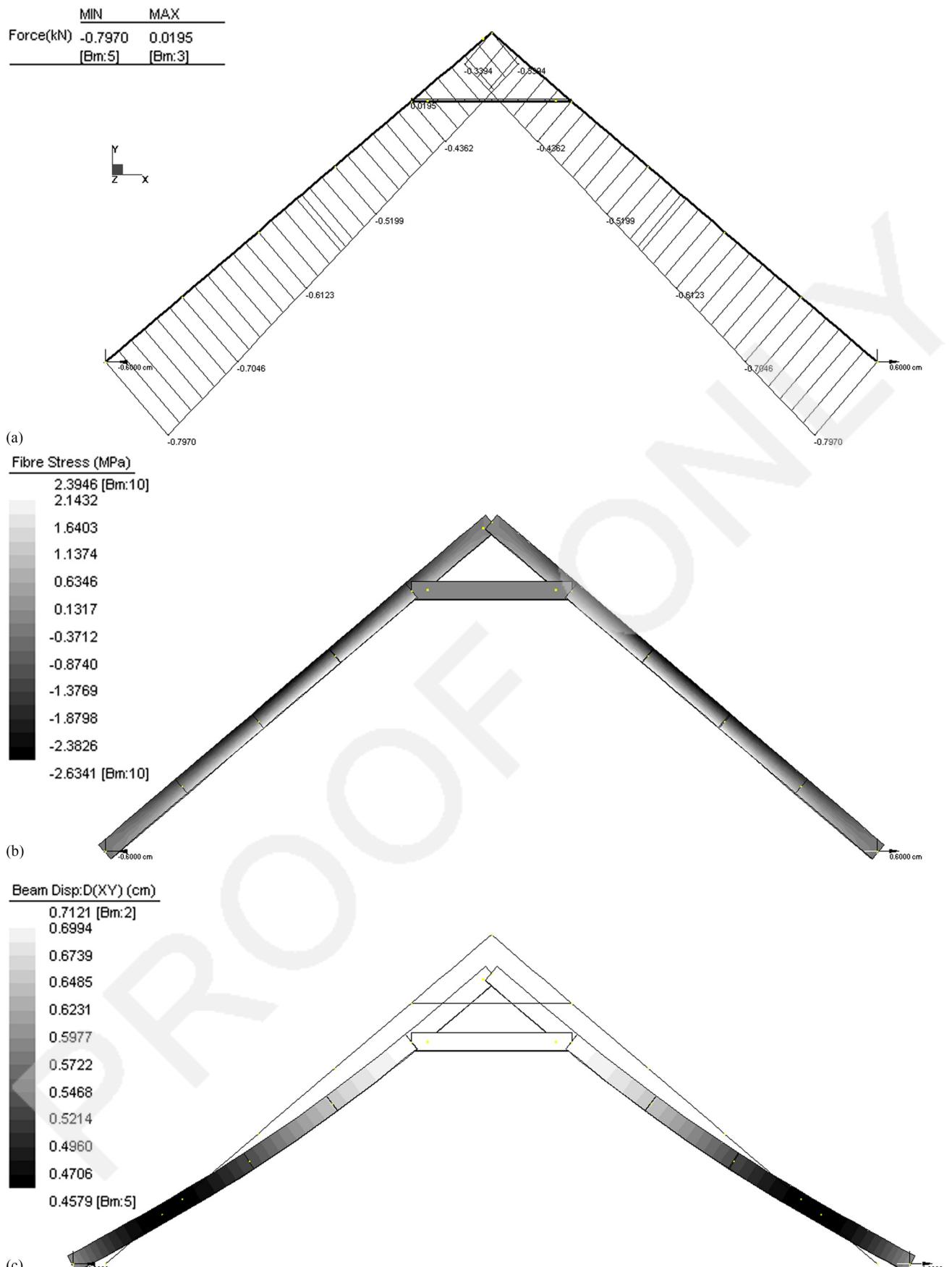


(c)

F8 : 1 **Fig. 8.** FEM\_1 Model—statically determined structure joined to the walls by a pinned and a roller support: (a) axial load; (b) stress state; F8 : 2 (c) horizontal and vertical node displacements.



F9 : 1 **Fig. 9.** FEM\_2 Model—statically indeterminate structure joined to the walls with pinned supports: (a) axial load; (b) stress state; (c) horizontal and vertical node displacements.  
 F9 : 2



F10 : 1 **Fig. 10.** FEM\_3 Model—statically indeterminate structure joined to the walls by pinned supports subjected to inelastic displacements of approximately 0.6 cm: (a) axial load; (b) stress state; (c) horizontal and vertical node displacements.

- 432 • The horizontal beams arranged transversally to the room are  
 433 not connected to the longitudinal edge beams.  
 434 • There is a lack of actual tie-beams.  
 435 • The connection between each pair of rafters in the longitudinal  
 436 direction relies only on horizontal and inclined planks wedged  
 437 into grooves carved in the rafters and does not realize a per-  
 438 fectly three-dimensional (3D) structure behavior.  
 439 • The node between the rafters and the edge beams is obtained  
 440 through a simple cavity hollowed out on the upper corner of  
 441 the edge beams where the rafters are inserted.  
 442 • The connection between the roof structure and the walls at the  
 443 level of the longitudinal beams that are simply placed (as  
 444 sleeper beams) on the top of the walls without fasteners, which  
 445 does not offer the possibility of providing a joint with a higher  
 446 level of safety, is yet, however, sufficiently efficient in the  
 447 examined context. In fact, even if the inevitable small hori-  
 448 zontal displacement toward the outside of the edge beam on the  
 449 wall is confirmed by a horizontal crack that is visible on the  
 450 outside wall of the room of the *ghorfa* at the roof-wall inter-  
 451 face [Fig. 12(a)], it nevertheless does not seem to put the over-  
 452 all stability of the system at risk. In particular, it is worth  
 453 highlighting that the masonry typology of the building is a  
 454 mixed stone and brick masonry, bound with lime–earth mortar,  
 455 that does not provide an efficient monolithic behavior due to  
 456 both the hard and scarcely hewing stones and the poor mortar  
 457 with a low amount of lime. Furthermore, as reported by  
 458 Rovero and Fratini (2013), the average values of the mech-  
 459 anical properties of this masonry are rather low: compressive

460 strength 2.9 N/mm<sup>2</sup>, Young modulus 1,340 N/mm<sup>2</sup>, shear  
 461 stress approximately 0.05 N/mm<sup>2</sup>.

462 This type of damage, in fact, has been acknowledged and high-  
 463 lighted by the most recent Moroccan regulations, *Reglement para-*  
*464 sismique des constructions en terre (RPCT 2011)* [Fig. 12(b)]. The  
 465 same regulations suggest, in fact, some reinforcement interventions  
 466 aimed precisely at improving the connection between the edge  
 467 beam of the Andalusian-type roof and the walls of the room  
 468 (Fig. 13).

## Seismic Vulnerability Assessment

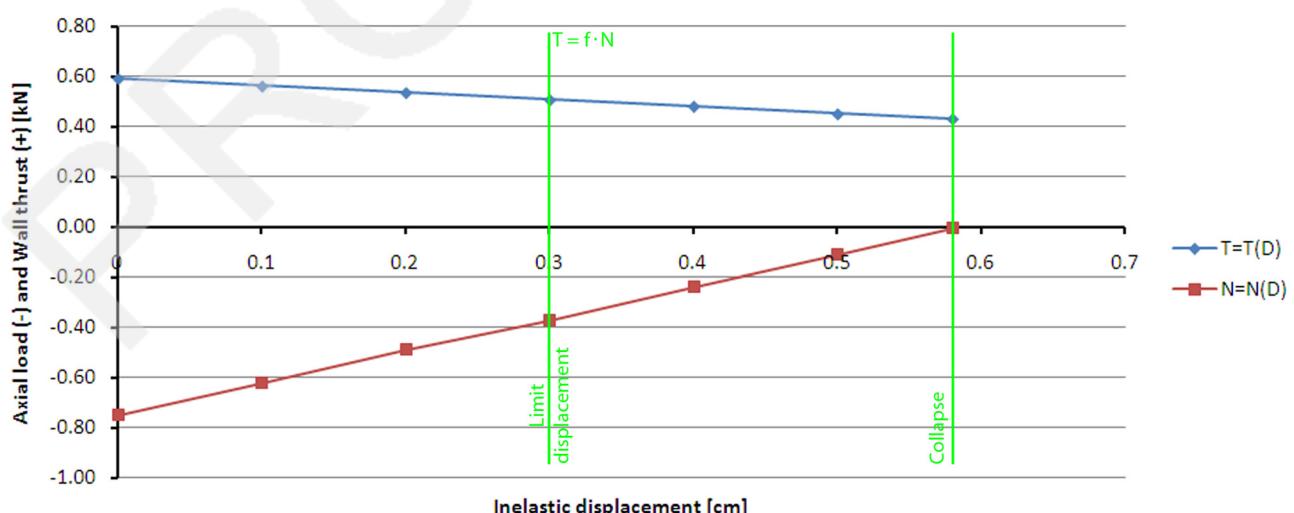
469 The state of unstable equilibrium based on friction, discussed pre-  
 470 viously, which was ascertained under the assumption of only  
 471 gravitational loads and the value of 0.7 for the static coefficient of  
 472 friction, could also be overestimated, and the structure could be,  
 473 instead, in a condition of higher risk because of the arbitrariness  
 474 with which this coefficient can be assumed. In fact, according to  
 475 the technical literature, reference to the dynamic coefficient of  
 476 friction of 0.25 at the timber–masonry interface, which is less  
 477 than half of that considered, should be made in the case of an  
 478 earthquake (Rizvi 2005). This reduced value is due to the seismic  
 479 actions that provoke the relevant vibrations of the structure. This  
 480 is the main reason why the Moroccan-type Andalusian timber  
 481 roof, as-is, cannot be considered safe with respect to possible  
 482 earthquakes.

483 Therefore, to assess the seismic vulnerability level of the  
 484 Andalusian roof, reference was made to a structural model where  
 485 all the rafters were considered as perfectly pinned at the base, there-  
 486 fore assuming a theoretical condition of poststrengthening so as to  
 487 prevent any displacement, in accordance with the RPCT (2011) rec-  
 488 commendations. Under this assumption, reference to the dynamic  
 489 coefficient of friction is omitted in this article because the sliding  
 490 failure is considered prevented by fasteners.

491 For this reason, an additional analysis was carried out with the  
 492 creation of a 3D model (FEM\_4) to assess the response of the roof  
 493 structure when subjected to a seismic action (Pugi and Galassi  
 494 2013). This model included both the load-bearing elements of the  
 495 structural system, using monodimensional elements of the beam  
 496 type and the wooden deck that supports the covering of the roof  
 497 through plane plate-type elements.

**Table 4.** Parametric analysis: Wall thrust and collar beam axial load as a function of the inelastic displacement of the supports

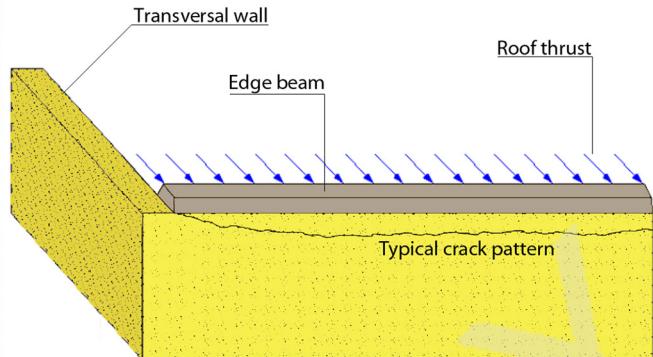
Inelastic displacement (cm)	Wall thrust (kN)	Collar beam axial load (kN)
0	0.59	-0.75
0.10	0.56	-0.62
0.20	0.53	-0.49
0.30	0.51	-0.37
0.40	0.48	-0.24
0.50	0.45	-0.11
0.58	0.43	-0.0062



**Fig. 11.** Results of the parametric analysis: wall thrust  $T$  and collar beam axial load  $N$  as a function of the inelastic displacement  $D$  of the supports.



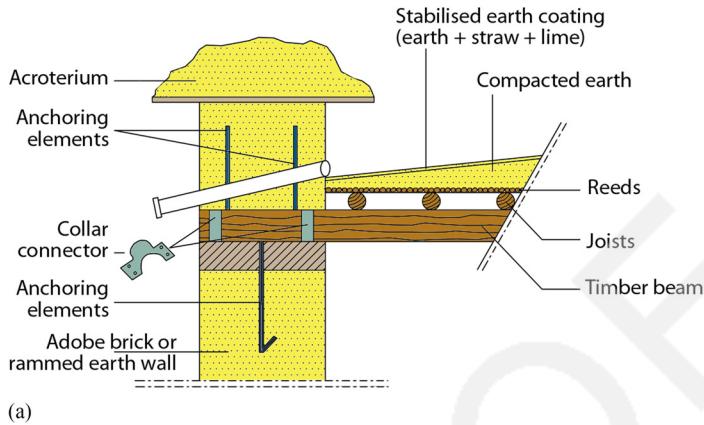
(a)



(b)

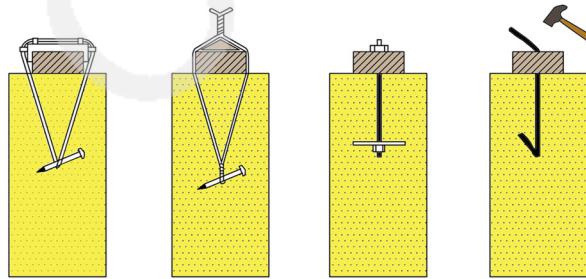
F12 : 1 **Fig. 12.** Horizontal crack on the wall due to the horizontal displacement of the roof's support beam: (a) *ghorfa* of Raissouni *dar* (image by Letizia Dipasquale); (b) crack scheme (adapted from RPCT 2011).

F12 : 2



(a)

Types of joints timber-masonry



(b)

F13 : 1 **Fig. 13.** (a) Reinforcement intervention for improving the connection between the edge beam of the roof and the wall of the room; (b) types of joints of the edge beam to the wall. (Adapted from RPCT 2011.)

F13 : 2

499 Two load conditions, in addition to the one due to gravitational  
500 loads, were formulated: seismic action in the transversal direction  
501 (*X*-direction) and seismic action in the longitudinal direction  
502 (*Z*-direction).

503 To compute the seismic action, reference was made to the  
504 Moroccan *Règlement de Construction Parasismique RPS 2000—*  
505 *Version 2011* (RPS 2000). According to these regulations, the  
506 national territory of Morocco is divided into five homogeneous seis-  
507 micity zones (from 0 to 4) that present approximately the same level  
508 of seismic risk, with a probability equal to 10% in 50 years for the  
509 recurrence of a seismic event.

510 The probability of 10% in 50 years was adopted by the regula-  
511 tions envisaging a seismic event of medium intensity that can occur  
512 several times during the life span of a structure. Fig. 14 presents the  
513 map of the seismic areas of Morocco and shows the location of  
514 Chefchaouen.

515 The city of Chefchaouen is situated in homogeneous Zone 3,  
516 which is characterized by an expected seismic acceleration equal to  
517 0.18g.

518 The combined effect of gravitational loads with the horizontal  
519 seismic action evaluated as an equivalent static force, in perfect ac-  
520 cordance with the provisions of the Moroccan regulations, was

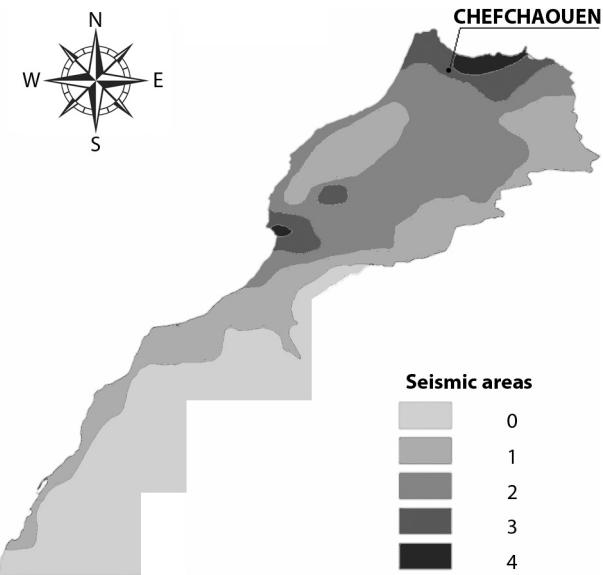
521 considered. In particular, the seismic action was inserted into the  
522 model by applying an additional load condition with a horizontal  
523 acceleration equal to  $0.18g = 176.58 \text{ cm/sec}^2$ .

524 The analysis of this fourth model (Fig. 15) clearly highlights that  
525 the seismic action does not significantly increase the thrust trans-  
526 mitted to the wall, nor does the axial load on the collar beam.  
527 Therefore, considering an earthquake of medium intensity but with  
528 a high probability of occurrence, the Andalusian-type roof shows a  
529 good level of safety with respect to the possibility of seismic events.

530 It is worth highlighting that the axial compression load on the  
531 collar beam obtained in the 3D model ( $-0.36 \text{ kN}$ ) is approximately  
532 half of that obtained in the 2D model. This is due to the presence of  
533 plate elements, inserted to simulate the deck of boards placed as loz-  
534 enges studded to the extrados of the elements of the roof, which evi-  
535 dently increases the stiffness of the overall structure.

536 Furthermore, the stress state is very low and therefore is not cap-  
537 able of putting the structure at risk [Fig. 15(b)].

538 The longitudinal effect of the seismic action has an even lesser  
539 influence because the structure presents a great longitudinal stiffness  
540 due to the dense repetitiveness of the timber elements that constitute  
541 the structural system of the roof, made even more efficient by the re-  
542 ciprocal connection carried out by the continuous deck of boards.



F14 : 1 **Fig. 14.** Map of the homogeneous seismicity zones of Morocco as a function of the peak accelerations. (Adapted from RPS 2000.)  
 F14 : 2

543 The results of the analysis for gravitational loads and seismic  
 544 action are presented in Table 5.

545 Lastly, it is necessary to note that, in the authors' opinion, the  
 546 strengthening interventions proposed by the Moroccan regulations  
 547 consisting of the use of fasteners to anchor the timber roof (i.e., the  
 548 edge beams) to the walls is effective only in the case of good-  
 549 quality masonry, capable of supporting both the horizontal and vertical  
 550 thrust provided by the roof. However, in traditional buildings  
 551 of the medina, such as in the case of Raissouni *dar*, the walls are often  
 552 made of irregular blocks laid down without shaping due to the  
 553 hardness and assembled with clay mortar. For this reason, despite  
 554 the strength of the stone (which is relatively high), the overall  
 555 strength of the masonry is not high because it is not guaranteed a  
 556 structure capable of stress uniformity or of monolithic behavior  
 557 (Rovero and Fratini 2013). Therefore, because the seismic vulner-  
 558 ability of the roof is a function of the anchorage of the roof itself to  
 559 the walls, which can improve the seismic response of the whole  
 560 building, the authors are convinced that the proposal of the building  
 561 regulations is not quite adequate and that, instead, the better way to  
 562 provide anchorage of the roof structure to the wall could be to link  
 563 the edge beams to the transversal beams of the attic. In this way, the  
 564 horizontal thrust, which, regardless of seismic action, is yet pro-  
 565 voked by the gravitational loads due to the heavy screed over the  
 566 deck planking, can be nullified, and the shear failure or the over-  
 567 turning of the walls can be prevented.

## 568 Conclusions

569 This article presents an in-depth analysis of the structural system of  
 570 the Andalusian-type roof for the courtyard house, a typical building  
 571 typology in northern Morocco. The analyses have been performed  
 572 using numerical simulations with both 2D and 3D models carried  
 573 out with the finite-element software *Straus7*.

574 In particular, the role that each structural element plays within  
 575 the roof system to ensure its equilibrium was highlighted using 2D  
 576 models. At the same time, it was possible to ascertain that the build-  
 577 ing system presents some inherent vulnerabilities due to the

particular building technique adopted, which relies on unilateral-type joints and elements and on frictional supports.

These vulnerabilities, however, have proven to be not significant if the Andalusian structure is constructed in the context of the Moroccan territory because the climate conditions do not make it probable to have important increases in terms of load due to snow.

However, the simplicity and the typology of the connection joints among the elements of the roof structure provoke a thrust on the perimeter masonry walls that cannot be prevented. The thrust cannot be entirely balanced by the support reaction exerted in correspondence to the interface between the edge beam and the masonry wall, which is only based on the friction force. Therefore, a slight horizontal sliding toward the outside inevitably occurs. The visible horizontal damage on the external wall at the level of the connection between wall and roof clearly shows the aforementioned phenomenon.

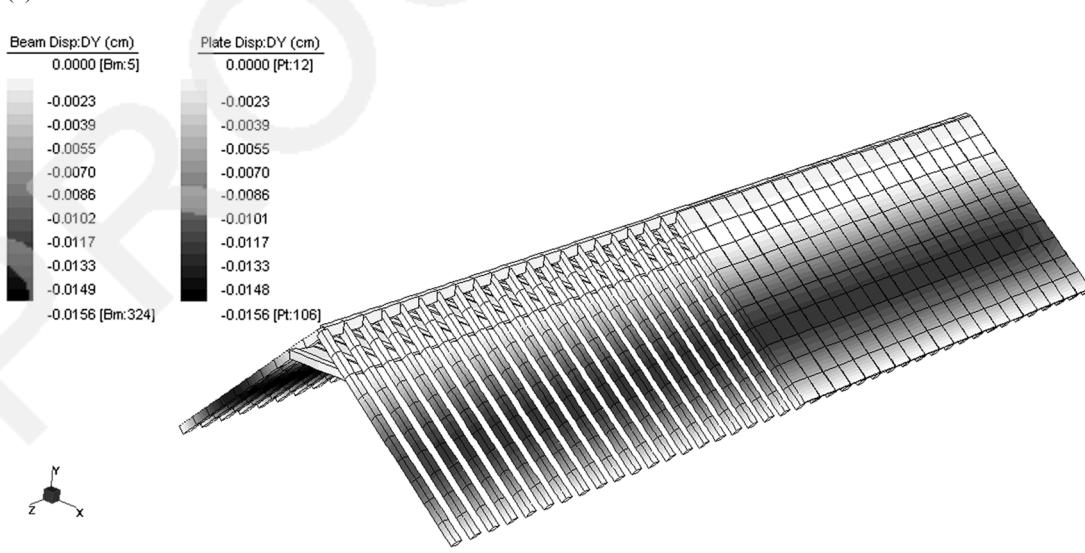
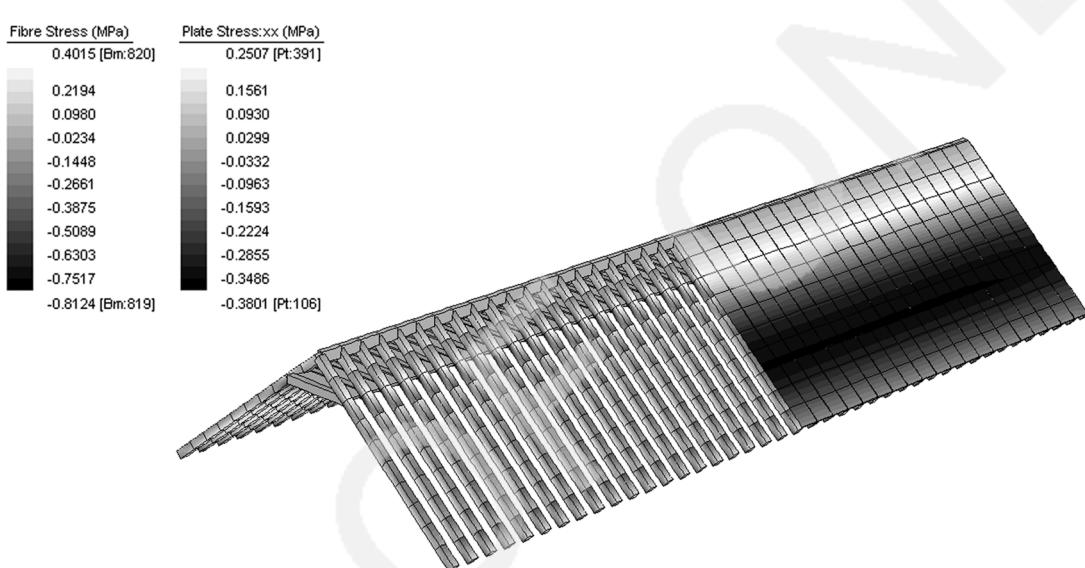
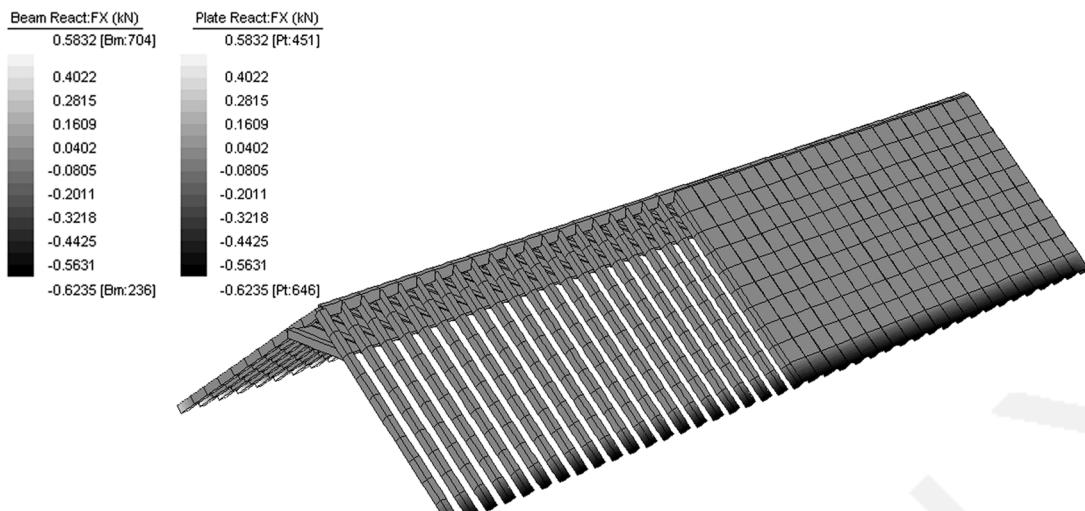
Local building regulations codified this type of crack as a recurring type of damage in Moroccan buildings where the Andalusian-type roof is used and indicate possible and specific techniques for reinforcing and improving the joint. Taking inspiration from the recommendations of the local building regulations, which propose devices for the anchorage of the roof to the walls, the seismic analysis was performed considering pinned supports that cannot move in a 3D model. It has been demonstrated that the effects of a seismic action, with levels of intensity predicted by the regulations, are not capable of modifying, in any significant way, the equilibrium and stability of the structure.

However, because the authors do not share the anchorage device proposed by the Moroccan regulations, to prevent the thrust of the roof on the masonry walls and reduce the seismic vulnerability, a strengthening intervention based on an effective connection between the transversal and the edge beams was proposed. Indeed, as mentioned previously, the actual seismic vulnerability depends on the anchorage of the roof to the walls, which is affected by the geometric and mechanical characteristics of masonry, which, in the specific case, has proven not to provide, in any way, a monolithic behavior or a high strength.

Finally, a parametric analysis was carried out to compute the limit value of the horizontal displacement that can turn the structure into a mechanism. The analysis highlighted the fact that the limit value is never actually reached in the case of gravitational loads. In fact, the horizontal displacement, once it has begun, stops when the thrust on the edge wall decreases and is balanced by the timber–masonry friction force of supports, as a consequence of the new configuration of the structure due to the displacement itself.

In this regard, it is worth noting that this article is a first contribution to the knowledge of the structural behavior of the Moroccan Andalusian timber roof. A fourth-step analysis based on an additional numerical model that also takes into account the masonry walls could be a very realistic analytical simulation to be performed to provide a more in-depth understanding of the behavior of each timber element of the roof structure. But, as might be expected, in this model, the behavior of the collar beam (i.e., if subject to compression or tensile axial load) would be the consequence of the deformability of the wall rather than the rigid sliding of the edge beam on the walls, whereas, instead, the horizontal crack detected on the wall exactly under the edge beam of the roof has clearly proven a rigid-cracking behavior of masonry due to the sliding failure. Because such a model would need a more accurate assessment of the mechanical properties of both timber and masonry, in addition to general knowledge regarding the geometric features of specific analyzed buildings, this issue will be addressed in a further study.

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F15 : 1 **Fig. 15.** FEM\_4 Model—3D model of the Andalusian-type roof for seismic analysis. Results regarding the gravitational load combination with the seismic action in the transverse direction: (a) thrust transmitted to the walls; (b) stress state; (c) vertical displacements.  
F15 : 2

**Table 5.** Summary of the results of the analysis of the Andalusian-type roof for gravitational loads and seismic action

Thrust transmitted to the wall versus collar beam axial load	Load Combination 1 (gravitational loads)		Load combination 2 (gravitational loads + Earthquake X)		Load combination 3 (gravitational loads + Earthquake Z)	
	Edge frame (kN)	Central frame (kN)	Edge frame (kN)	Central frame (kN)	Edge frame (kN)	Central frame (kN)
In plane thrust ( $X^a$ )	0.34	0.6	0.36	0.62	0.37	0.6
Vertical thrust ( $Y^b$ )	0.44	0.78	0.46	0.79	0.47	0.78
Out-of-plane thrust ( $Z^c$ )	0.0047	0	0.0042	0	0.013	0.023
Collar beam axial load	-0.3	-0.37	-0.3	-0.36	-0.31	-0.37

Note: Medium-intensity earthquake as defined by RPS (2000).

<sup>a</sup>Room transversal direction.

<sup>b</sup>Gravitational load direction.

<sup>c</sup>Room longitudinal direction.

For the aforementioned reasons, the authors will use their specific software *BrickWORK* (Galassi and Paradiso 2014; Galassi and Tempesta 2018), already used to perform the analysis of masonry constructions in earlier works (Paradiso et al. 2013, 2014a, b) and suitably developed to model the walls by rigid blocks, even assembled with heart-based mortar joints that are characterized by an elastic behavior under compressive forces and a rigid-cracking behavior under tensile forces, in agreement with the effective performance of the masonry that has proven not to provide a monolithic behavior. The results will also be compared to those provided by the use of *Straus7*, herein used to perform the analyses. It is expected to realistically describe the effect of the spreading roof on the side masonry walls and, therefore, the overall behavior of the roof–wall structure.

## References

- Anderson, G. D., and M. Rosser-Owen. 2007. *Revisiting al-Andalus: Perspectives on the material culture of Islamic Iberia and beyond*. Leiden, Netherlands: Brill.
- Blau, P. J. 1996. *Friction science and technology*. Boca Raton, FL: CRC.
- Candelas Gutiérrez, A. L. 2003. "On the origin of some 'whiteness' carpentry rules." In *Proc., 1st Int. Congress on Construction History*, edited by S. Huerta, 505–512. Madrid: Instituto Juan de Herrera.
- CEN (European Committee for Standardization). 2002. *Actions on structures, part 1.1*. Eurocode 1. Brussels, Belgium: CEN.
- Copani, P. 2006. "Ancient wooden roofs in Sicily: The heritage of Taormina." In *Proc., 2nd Int. Congress on Construction History*, 817–835. Exeter, UK: Short Run.
- Cruz, H., D. Yeomans, E. Tsakanika, N. Macchioni, A. Jorissen, M. Touza, M. Mannucci, and P. B. Lourenço. 2015. "Guidelines for on-site assessment of historic timber structures." *Int. J. Archit. Heritage* 9 (3): 277–289. <https://doi.org/10.1080/15583058.2013.774070>.
- Dipasquale, L., S. Mecca, L. Rovero, and U. Tonietti. 2008. "Technical knowledge and traditional architecture in medina of Chefchaouen (Morocco)." In *Proc., 2nd Int. Conf. on Mediterranean Heritage (RIPAM)*. Marrakech, Morocco: Université Cadi-Ayyad.
- Dipasquale, L., and V. Volpi. 2009. "Le coperture." In *Chefchaouen. Architettura e cultura costruttiva*, edited by S. Mecca, L. Dipasquale, L. Rovero, U. Tonietti, and V. Volpi, 167–181. Pisa, Italy: Edizioni ETS.
- Du Bois, A. J. 1902. *The mechanics of engineering*. New York: Wiley.
- Elert, G. 2017. "Friction." *The physics hypertextbook*. <https://physics.info/friction>.
- Galassi, S., and M. Paradiso. 2014. "BrickWORK software-aided analysis of masonry structures." *IERI Procedia*, 7: 62–70. <https://doi.org/10.1016/j.ieri.2014.08.011>.
- Galassi, S., M. Paradiso, and G. Tempesta. 2013. "Non-linear analysis of masonry structures subjected to external settlements." *Open J. Civ. Eng.* 3 (2A): 18–26. <https://doi.org/10.4236/ojce.2013.32A003>.
- Galassi, S., and G. Tempesta. 2018. "Analysis of masonry block structures with unilateral frictional joints." In *Proc., 2nd Int. Conf. on Materials, Manufacturing and Mechanical Engineering (MMME2017)*. Dubai, United Arab Emirates: Emirates Research Publishing.
- Gorst, N. J. S., S. J. Williamson, P. F. Pallett, and L. A. Clark. 2003. *Friction in temporary works*. Research Rep. 071 Prepared for Health and Safety Executive. Birmingham, UK: Univ. of Birmingham, School of Engineering.
- Grigoriev, I. S., and E. Z. Meilikhov. 1997. *Handbook of physical quantities*. Boca Raton, FL: CRC.
- Lee, S.-H., S. Harari, H. Wong, and V. Kapila. 2005. "Friction experiment." *SMART—Science and Mechatronics Aided Research for Teachers 2003–2005*. <http://engineering.nyu.edu/mechatronics/smart/pdf/experiments/Project3.pdf>.
- Mastrodicasa, S. 1948. *Disseti statici delle strutture edilizie. Diagnosi e consolidamento*. Milano, Italy: Hoepli.
- Murase, Y. 1984. "Friction of wood sliding on various materials." *J. Facil. Agric. Kyushu Univ.* 28 (4): 147–160.
- Nuere, E. 1989. *La carpintería de armar española. Editores instituto de conservación y restauración de bienes culturales*. Madrid, Spain: Ministerio de Cultura, Instituto de Conservacion y Restauracion de Bienes Culturales.
- Orlando, M., L. Salvatori, P. Spinelli, and M. De Stefano. 2016. "Displacement capacity of masonry piers: parametric numerical analyses versus international building codes." *Bull. Earthquake Eng.* 14 (8): 2259–2271. <https://doi.org/10.1007/s10518-016-9903-x>.
- Paradiso, M., S. Galassi, A. Borri, and D. Sinicropi. 2013. "Reticulatus: An innovative reinforcement for irregular masonry. A numeric model." In *Structures and architecture: Concepts, applications and challenges*, edited by P. J. S. Cruz. Boca Raton, FL: CRC.
- Paradiso, M., S. Galassi, and D. Sinicropi. 2014a. "Il caratteristico Forte di San Fernando de Bocachica, Colombia." In *Proc., Congress on Construction Pathology, Rehabilitation Technology and Heritage Management (Rehabend 2014)*, edited by L. Villegas, I. Lombillo, C. Liano, H. Blanco. Santander, Spain: Gráficas Iguña.
- Paradiso, M., S. Galassi, and D. Sinicropi. 2014b. "Stabilità di archi e volte in muratura: Metodi tradizionali e calcolo automatico." In *Proc., Congress on Construction Pathology, Rehabilitation Technology and Heritage Management (Rehabend 2014)*, edited by L. Villegas, I. Lombillo, C. Liano, H. Blanco. Santander, Spain: Gráficas Iguña.
- Parisi, M. A., and M. Piazza. 2000. "Mechanics of plain and retrofitted traditional timber connections." *J. Struct. Eng.* 126 (12): 1395–1403. [https://doi.org/10.1061/\(ASCE\)0733-9445\(2000\)126:12\(1395\)](https://doi.org/10.1061/(ASCE)0733-9445(2000)126:12(1395)).
- Parisi, M. A., and C. Chesi. 2014. "Seismic vulnerability of traditional buildings: The effect of roof-masonry walls interaction." In *Proc., 10th US National Conf. on Earthquake Engineering*, Oakland, CA: Earthquake Engineering Research Institute.
- Parisi, M. A., M. Piazza, and C. Chesi. 2011. "Seismic response of traditional timber elements and roof structures: Learning from the L'Aquila earthquake." In *Proc., 11th Int. Conf. Structural Health Assessment of Timber Structures*, 1–9. Lisbon, Portugal: Laboratorio Nacional de Engenharia Civil.
- Pugi, F., and S. Galassi. 2013. "Seismic analysis of masonry voussoir arches according to the Italian building code." *Ing. Sism.* 3: 33–35.

- 743 Rizvi, M. A. 2005. *Effective technical communication*. New Delhi, India: 759  
744 Tata McGraw-Hill Education. 760
- 745 Rovero, L., and F. Fratini. 2013. "The medina of Chefchaouen (Morocco): 761  
746 A survey on morphological and mechanical features of the masonry." 762  
747 *Constr. Build. Mater.* 47: 465–479. <https://doi.org/10.1016/j.conbuildmat.2013.05.025>. 763
- 748 RPCT (Royaume du Maroc, Ministère de l'Interieur, Ministère de 764  
750 l'Equipement et du Transport, Ministère de l'Habitat, de l'Urbanisme et 765  
751 de la Politique de la Ville). 2011. *Règlement parassismique des construc- 766  
752 tions en terre*. RPCTerre 2011. Rabat, Morocco: RPCT. 767
- 753 RPS (Royaume du Maroc, Ministère de l'Habitat et de la Politique de la 768  
754 Ville). 2000. *Règlement de Construction Parassismique*. RPS 2000— 769  
755 Version 2011. Decreto n. 2-02-177 du 9 hija 1422 (22 febbraio 2002). 770  
756 Djibouti, Africa: RPS. 771
- 757 Ruggieri, N., S. Galassi, and G. Tempesta. 2018. "Pompeii's Stabian baths. 772  
758 Mechanical behaviour assessment of selected masonry structures during 773  
774 the 1st century seismic events." *Int. J. Archit. Heritage*. <https://doi.org/10.1080/15583058.2017.1422571>. 775
- Tampone, G. 2001. "Acquaintance of ancient timber structures." In *Proc., 3rd 761  
762 Seminar on Structural Analysis of Historic Constructions*, edited by P. B. 762  
763 Lourenço and P. Roca, 117–143. Guimaraes, Portugal: Univ. of Minho. 763
- Tampone, G. 2005. "Il complesso lignario della Cattedrale di Nicosia." 764  
In *Proc., Int. Conf. on Conservation of Historic Wooden Structures*, 765  
144–154. Florence, Italy: Alinea. 766
- Tampone, G., and N. Ruggieri. 2016. "State-of-the-art technology on con- 767  
servation of ancient roofs with timber structure." *J. Cult. Heritage*, 22: 768  
1019–1027. <https://doi.org/10.1016/j.culher.2016.05.011>. 769
- UNI (Ente Nazionale Italiano di Unificazione). 2009. *Structural timber. 770  
Strength classes*. EN 338:2009. Brussels, Belgium: UNI. 771
- UNI (Ente Nazionale Italiano di Unificazione). 2010. *Structural timber. 772  
Visual strength grading. Part 3: Timber Uso Fiume and Uso Trieste*. 773  
UNI 11035-3:2010. Milano, Italy: UNI. 774

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