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## Article

# Application and Evaluation of a Non-Accident-Based Approach to Road Safety Analysis Based on Infrastructure-Related Human Factors

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**Abstract:** Too often the identification of critical road sites is made by “accident-based” methods that consider the occurred accidents’ number. Nevertheless, such a procedure may encounter some difficulties when an agency does not have reliable and complete crash data at the site level (e.g., accidents contributing factors not clear or approximate accident location) or when crashes are underreported. Furthermore, relying on accident data means waiting for them to occur with the related consequences (possible deaths and injuries). A non-accident-based approach has been proposed by PIARC. This approach involves the application of the Human Factors Evaluation Tool (HFET), which is based on the principles of Human Factors (HF). The HFET can be applied to road segments by on-site inspections and provides a numerical performance measure named Human Factors Scores (HFS). This paper analyses which relationship exists between the results of the standard accident-based methods and those obtainable with HFET, based on the analysis of self-explaining and ergonomic features of the infrastructure. The study carried out for this purpose considered 23 km of two-way two-lane roads in Italy. A good correspondence was obtained, meaning that high risky road segments identified by the HFS correspond to road segments already burdened by a high number of accidents. The results demonstrated that the HFET allows for identifying of road segments requiring safety improvements even if accident data are unavailable. It allows for improving a proactive NSS, avoiding waiting for accidents to occur.

**Keywords:** road safety; human factors; network safety screening; proactive procedure



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## 1. Introduction

Road traffic injuries represent a major public health problem, and they are still a leading cause of death, injury, and disability worldwide. Every year, nearly 1.35 million people die because of road crashes, and another 20 to 50 million sustain serious injuries resulting in long-term adverse health consequences. Road traffic injuries are the eighth leading cause of death for people of all ages and the first leading cause of death for children and young adults (5–29 years) [1]. Furthermore, the death rate in low-income countries is three times higher than in high-income countries. The number of road traffic deaths continues to rise.

The European Road Safety Policy Orientations 2020–2030 [2] proposed a set of initiatives to improve and expand the application of the European Union (EU) Directive 2008/96 CE [3] and build safer road infrastructures. The initiatives include setting higher standards for vehicle safety, improving road user training and compliance, and increasing road rules enforcement. Society, governments, and road authorities (RAs) are now facing the problem and have taken actions to reduce the number of accidents, death, and serious injuries. Road safety analysis procedures are being improved [4,5]. According to one of the safe System’s five pillars, roads must be safe [6]. This means that road design must consider safety as a first option. To implement effective countermeasures, road authorities need methodologies and tools capable of assessing the potential safety issues and understanding

which segments to prioritize due to increasingly less available resources. For this reason, Network Safety Screening (NSS) procedures must be implemented.

As stated in the FHWA-SA-16-037 report by Srinivasan et al. [7], the NSS “is the process of identifying sites for further investigation and potential treatment. The intent is to identify sites expected to benefit the most from targeted, cost-effective treatments . . .”. Conducting an effective NSS is the best way to implement a correct investment plan and improve road safety by preventing the occurrence of road accidents. To identify road segments candidates for safety improvements, one of the most used approaches is the identification of high accident concentration segments. This approach is widely used by many RAs to identify where to invest money and resources [8]. This accident-based approach belongs to the group of reactive approaches: something is done only after accidents have occurred. Identifying road hazardous segments using accident analysis is effective when large and reliable accident datasets are available. The analysis of the number of observed accidents helps to define numerical performance measures (such as accident frequency, accident density, and accident rate) that can be used to make a Network Safety Ranking [4], which is the follow-up of an NSS. However, accident-based approaches show four main limitations: accidents need to occur; accidents are still a stochastic variable; data analysis issues are present, such as regression to the mean phenomenon (mainly if a short analysis period is considered) and non-linear relationship between traffic and accidents [7]; and finally, accident data are not always available (e.g., there is a high lack of data in Low- and Medium-Income Countries) [9] or are incomplete (even in high-income countries, e.g., the Italian dataset shows some issues such as inaccurate accident localization and poor location description, or not recorded property-damage-only accidents). In the literature, other approaches have been proposed. One widely used method that represents the best available tool for analyzing potential safety issues is the use of Accident Prediction Models (APMs) in conjunction with an Empirical Bayesian (EB) methodology. APMs account for the systematic influence of road layout on accident occurrence and are developed by analyzing the historical accident trends occurring on similar road infrastructures using statistical procedures. These models allow relating of the number of expected crashes on a site to its specific geometric and environmental characteristics [10].

Nowadays, APMs have been extensively used in the road infrastructure field to estimate the number of expected accidents on road segments and junctions [11–13], as well as to determine the expected safety impacts of design changes [14–16]. Within a large number of APMs developed during the recent years, the HSM [17,18] and the PRACT Project [19] approaches offer a consistent method for making reliable crash frequency predictions. The APMs proposed are based on traffic, main road geometrical features (including planimetric, altimetric, and cross-sectional), and other functional and safety aspects, such as the presence of lighting or the presence of road safety barriers. The high reliability of these methods come at the price of a large and demanding amount of available data and reliable models: specific safety performance functions (SPFs) are sometimes required if the base conditions considerably differ from the standard conditions [20]. This places some limits on their use. Other NSS procedures are thus often necessary.

An innovative procedure to identify blackspots has been proposed by Wan et al. [21] following an EB procedure. Another example of the application of the EB procedure comes from the study by Yuan et al. [22]. In the latter study, one of the most influencing factors of side crashes at intersections was speeding. The understanding of speeding causes is strictly related to Human Factors, as those are factors that influence the driver’s perception of the road. Road safety analysis should always account for the influence of road characteristics on drivers’ behaviour.

Accidents occur due to the interaction between vehicle, road, and driver [17,23]. The interaction between the road and the driver is the leading cause of accident triggering factors [5,24,25]. Bend curvature perception [26,27], critical point perception and reaction time [28], influence of memory and road familiarity [29,30], influence of road elements on speed and behaviour [31–36], human workload and decision making [37], and influence of

societal attitudes on speed [38] are some of the issues that have been studied. After all, the driver's influence on road safety has been considered ever since the first car was driven, but only in the last decade has the topic been addressed by considering the influence of Human Factors in the right sense. PIARC (World Road Association) defines Human Factors as "those psychological and physiological threshold limit values which are verified as contributing to operational mistakes in machines' and vehicles' handling. In the case of road safety, the Human Factors concept considers road characteristics that influence a driver's right or wrong driving actions" [39]. The concept of Human Factors is very broad. It accounts for many aspects, such as drivers' compliance with the road rules, the societal influence [40], aggressive behaviour, the influence of gender and age on driver behaviour [41], how drivers interact with vehicle's controls [42], and the use of a telephone while driving. This research addressed a part of the overall Human Factors' aspects, which is the part that interests road engineers and road practitioners. That part is that underlined in the PIARC definition, where the infrastructure has its main role. This part is founded on concepts and rules from psychology, adopted in engineering to investigate, identify, and eliminate potentially dangerous situations.

All road locations (e.g., curves and intersections) must be visible and clear, the driver's field of view must unconsciously suggest the correct trajectory and the right speed, and the road layout should be consistent with the drivers' expectations. In other words, the road features should be self-explaining and ergonomic. A first decisive contribution to the analysis of the road environment and the concept of self-explaining roads comes from the research of Theeuwes and Godthelp [43–46], who stressed the influence of the road environment and the concept of a self-explaining road as an instrument to improve road safety. Nowadays, the concept of self-explaining roads has been widely accepted [47]. The image that drivers have of the road must be clear and the elements that allow a correct interpretation of the road must be visible and understandable. The influence of specific road environments on driver behaviour has been recently tested by Qin et al. [48]. To face the issue, the application of the Gestalt theory [49] to road design has been suggested in recent years [50–53].

The topic of the application of Human Factors to road safety has recently evolved in the setting of a practical evaluation tool, the Human Factors Evaluation Tool (HFET), to check the road features' coherence with Human Factors principles [54–56]. The HFET applicability and consistency in understanding road safety problems at critical locations was tested in Germany and Netherlands with good results [57,58] but was never considered for an NSS. In the German study, the HFET was used to assess which road issues are present in about 1500 accidents spots that seems to be "inexplicable". Some issues have been identified and solved using Human Factors principles. The tool was then tested in the European Project Rankers [57], where the researchers were asked to define the riskiest stretch among two. The prediction was right. In the Dutch study, a total of 26 pairs of locations were analyzed. One location of the pair was a black spot, while the other was a location with no or few accidents. The HFET researchers were asked to identify the critical location among those in the pair without knowing which was the black spot location. Their prediction rate was 75% successful, and it was statistically significant with an error probability of 5%. The HFET evaluates how much the road environment provides a positive answer to the Human Factors' demand. It is based on road safety inspection, and it provides quantitative results by calculating a safety performance measure called the Human Factors Score (HFS) [56].

In sum, NSS has been demonstrated to be a crucial process for RA. However, standard NSS relying on accident-based approaches (reactive) work well if a reliable and complete accident dataset exists. Unfortunately, too many countries do not have accident datasets to build reliable assessments, both in terms of APMs or observed accident-based performance measures. Moreover, standard NSS does not directly account for Human Factors aspects. On the other hand, proactive approaches allow for intervention before accidents occur. In this context, the HFET represents a methodology able to be proactive and prone to understand the relations between accident risk and Human Factors. The tool has never

been applied for an NSS, and similar studies accounting for Human Factors aspects in NSS seem to not be present in literature. The gap has been addressed by this research.

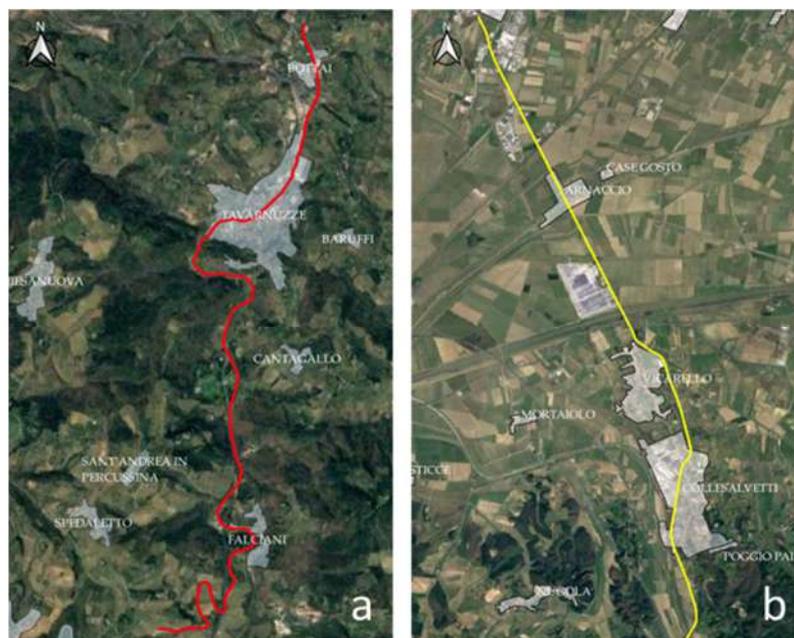
This paper shows the results of applying an NSS procedure based on Human Factors principles. The work was carried out as part of the CMRSS (Centro di Monitoraggio Regionale per la Sicurezza Stradale) research project of the Tuscan Region Administration. The research project aimed to provide RAs and road engineers with a practical, economical, and user-friendly decision-making tool for monitoring and screening of the entire road network. It was intended to analyze the safety meaning of a proactive methodology to carry out an NSS considering Human factors aspects through the application of the PIARC's innovative HFET [56]. To test the effectiveness of the procedure, the results were compared with those obtained by the application of a standard NSS procedure based on accident analysis. For the latter analysis, the expected number of accidents was considered, which was calculated using the Highway Safety Manual (HSM) predictive method [17] adjusted with an Empirical Bayes (EB) procedure.

The study carried out for this purpose considered 23 km of two-way two-lane roads in Italy, for which detailed traffic, accidents, and geometric data were available. The paper analyzed which relationship exists between the results of the standard accident-based methods and those obtainable with HFET, based on the analysis of self-explaining and ergonomic features of the infrastructure.

## 2. Materials and Methods

### 2.1. Test Roads

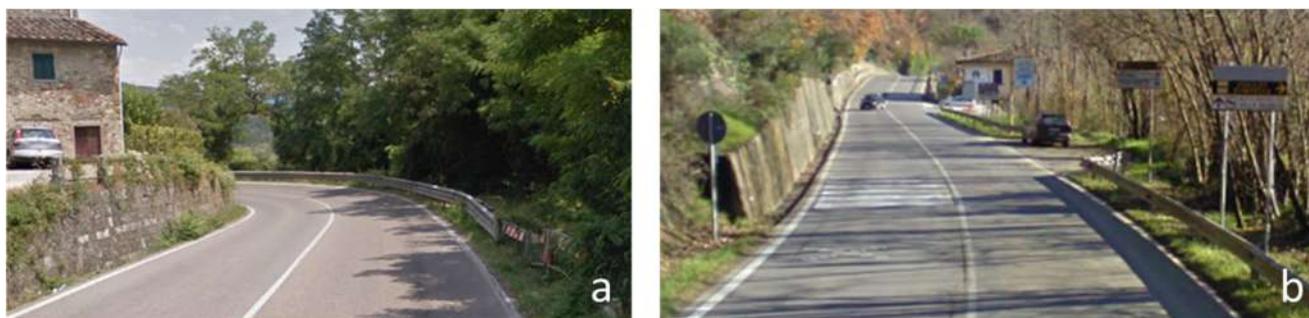
Two sections of two rural two-way two-lane roads, located in the Tuscan Region in Italy, were analyzed. These were the Strada Regionale 2 (SR2), from 281 + 600 km to 292 + 300 km, and the Strada Regionale 206 (SR206), from 29 + 600 km to 42 + 100 km. Figure 1 shows the two road stretches analyzed and their main geometrical features and differences.



**Figure 1.** The two analyzed road segments: SR2 (a) and SR206 (b). The Kilometers markers direction is northbound for both the roads.

The SR2 develops through a hilly stretch, characterized by many curves of a small radius, and it runs for a short stretch along the Greve river, maintaining a low curvature ratio. In the final part, the road passes through urban areas, reaching an important intersection junction with two roads of a higher functional class (two motorways). The road appears quite complex, with many curves of different radii and many driveways. The

average carriageway width is 6.95 m, with no or very narrow shoulders and close marginal elements. Figure 2 shows two views of the SR2. The first shows a bend (on hilly terrain) that is difficult to understand because neither its curvature nor its development is clear; the second shows a picture taken about 90 m before an intersection that is quite invisible because the point of attraction is the top of the subsequent crest, and some marginal elements cover the minor road.



**Figure 2.** Photos of SR2: 282 + 100 km northbound (a), and 288 + 400 km southbound (b).

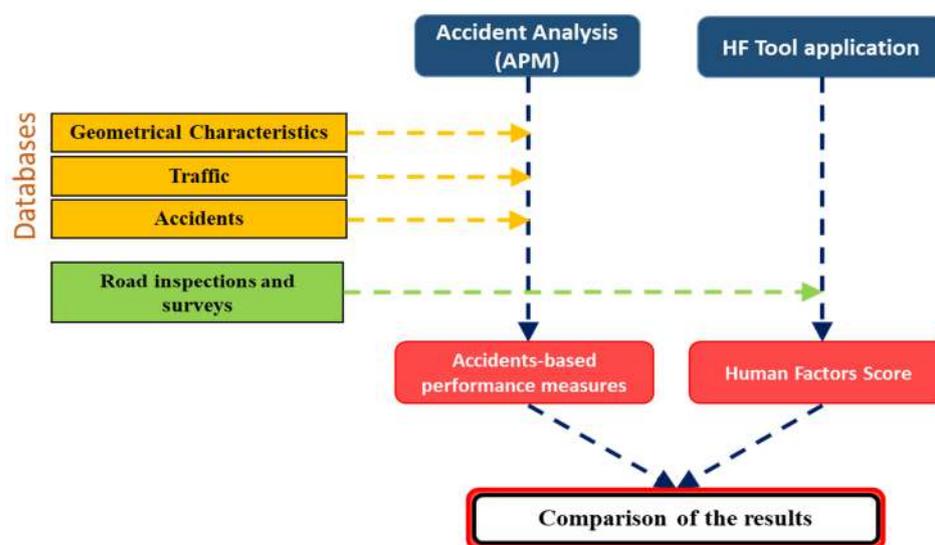
The SR206 develops in a flat environment and, overall, has a low curvature ratio, with a high presence of long straights and few intersections. The rural part of the road is generally clear and simple. The road passes through some small urban areas. There are many driveways along the road. The average carriageway width is 7.25 m, with 1 m paved shoulders on the central part of the road and very narrow paved shoulders on the remaining part. The left picture of Figure 3 shows a cross-section with shoulders of 1.5 m, while the one on right shows a cross-section with shoulders of about 0.5 m. The monotony of the margins is interrupted by some advertising panels, as shown in the right picture of Figure 3.



**Figure 3.** Photos of SR206: 33 + 700 km southbound (a), and 39 + 200 km northbound (b).

## 2.2. Procedure

The first activity concerned the segmentation process of the two considered test roads to subdivide them into homogeneous segments (HSs). According to the procedure described by PIARC [56], road safety inspections were performed for each homogeneous segment, and the Human Factors Score (HFS) was calculated. Finally, the HSM procedure was carried out, and each segment was assigned the expected number of accidents. The calculated HFS values and the accident-based performance measures of each segment were compared to assess their consistency. As shown in Figure 4, the HFET procedure requires less input data than the HSM analysis. Therefore, the procedure is also applicable in countries where a reliable accident, traffic, and geometrical database is not yet available.



**Figure 4.** Flow chart of the required data and steps implemented in the analysis.

### 2.3. Data Overview

Databases were provided by the Tuscan Region administration. They included the road characteristics, the traffic, and the accidents that occurred on the two test roads over 5 years (2014–2018). The road characteristics database considers a segmentation into segments with an average length of 100 m (ranging between 90 and 110 m). Each segment has an ID code, which refers to the name of the road and to the kilometers and hectometers markers they start from (e.g., the segment of SR2 starting from 234 + 300 km has the ID code SR2\_234\_3; the name of the road will be omitted if there is no risk of misunderstanding). The database contains the geometrical characteristics of each segment and specifies if they belong to the rural or urban environment. The segments and the location of each intersection were georeferenced.

The traffic database contains the traffic counts provided by inductive loops installed on the road network. In the test roads, there were four detection points: two on SR2 and two on SR206. The average traffic data along the considered road segments are shown in Table 1.

**Table 1.** Overall accidents and traffic parameters of the analyzed road segments.

Road	Analyzed Segment Length [km]	Mean Average AADT <sup>1</sup> [Veh/Days]	Max Average AADT <sup>1</sup> [Veh/Days]	Min Average AADT <sup>1</sup> [Veh/Days]	Total Accidents	Fatal Accidents	Injury Accidents
SR2	10.41	6469	12,395	4247	83	1	82
SR206	12.75	12,931	15,335	11,799	78	1	77

<sup>1</sup> Considering the mean/max/min of the average AADT of all segments. Average AADT is calculated as the average of the segment AADT in the analysis period.

The accidents database was provided by the ISTAT (Italian National Statistical Board) and by the Tuscany Region Administration. It contains information about severe (fatal and injuries) accidents. The accidents were all georeferenced. The number of accidents, deaths, and injuries during the analysis period (2014–2018) is given in Table 1. No data about property-damage-only (PDO) accidents were available. All accidents caused by non-infrastructure-related problems were discarded; this translated into a total of two discarded accidents because they were caused by a vehicle breakdown. No accidents related to alcohol or drug abuse were in the database.

#### 2.4. Test Roads Segmentation

The two test roads were subdivided into homogeneous segments (HSs) accounting for the road environment (urban and rural) and the type of road elements (roadway segments and intersections). The rural segments were divided into roadway segments and intersections segments. The urban segments were not divided. Each HS length ranged from 200 m to 1000 m. Their boundaries were rounded to fit the 100-meter-long segments of the available road characteristics database (see subchapter 2.3). The segmentation length was selected by accounting for the following factors:

- accident triggering factors: these are located far away from the crash location reported in the crash reports, depending on speed and accident dynamics. Too-short road segments could be burdened by accidents that were caused by road deficiencies of a preceding road segment;
- NSS management procedure: too-short road segments require high efforts in data collection and management. This has also been stated in the HSM [17], where the suggested minimum length is 0.1 mi (about 160 m);
- critical segments identification: too-long segmentation will not help the identification of critical locations.

The influence length of an intersection was 75 m from the intersection geometrical center [59]. If adjacent intersections were closer than 250 m, then the HS included all close-spaced intersections and enclosed road segments. Adjacent Urban Segments were considered together. The segmentation process results are summarized in Tables 2 and 3. A total of 47 HSs were obtained.

**Table 2.** Main characteristics of the HSs.

Type	Maximum Length [m]	Minimum Length [m]	Average Length [m]
Roadway Segments (rural)	908	200	490
Intersections (rural)	903	190	498
Urban Segments	707	296	599

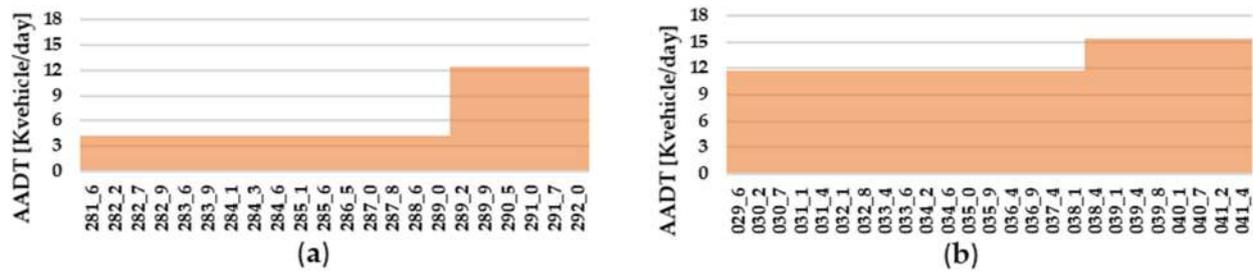
**Table 3.** HSs obtained for each road.

SR2		
Section ID	Length [km]	Section Type
281_6	0.60	Roadway Segment
282_2	0.50	Intersections
282_7	0.20	Roadway Segment
282_9	0.68	Intersections
283_6	0.29	Roadway Segment
283_9	0.20	Intersections
284_1	0.22	Roadway Segment
284_3	0.22	Intersections
284_6	0.43	Roadway Segment
285_1	0.50	Intersections
285_6	0.91	Roadway Segment
286_5	0.51	Intersections
287_0	0.81	Roadway Segment
287_8	0.80	Intersections

Table 3. Cont.

SR2		
Section ID	Length [km]	Section Type
288_6	0.40	Roadway Segment
289_0	0.20	Intersections
289_2	0.70	Urban Segment
289_9	0.60	Urban Segment
290_5	0.50	Urban Segment
291_0	0.55	Intersections
291_7	0.30	Urban Segment
292_0	0.35	Intersections
SR206		
Section ID	Length [km]	Section Type
029_6	0.60	Intersections
030_2	0.50	Roadway Segment
030_7	0.40	Intersections
031_1	0.30	Roadway Segment
031_4	0.70	Urban Segment
032_1	0.71	Urban Segment
032_8	0.58	Urban Segment
033_4	0.19	Intersections
033_6	0.58	Roadway Segment
034_2	0.39	Intersections
034_6	0.39	Roadway Segment
035_0	0.90	Intersections
035_9	0.49	Roadway Segment
036_4	0.69	Intersections
036_9	0.49	Intersections
037_4	0.69	Roadway Segment
038_1	0.30	Intersections
038_4	0.70	Urban Segment
039_1	0.30	Roadway Segment
039_4	0.40	Intersections
039_8	0.30	Roadway Segment
040_1	0.60	Intersections
040_7	0.50	Intersections
041_2	0.20	Roadway Segment
041_4	0.82	Intersections

Figure 5 shows the traffic distribution along the two road stretches analyzed. The obtained segments are listed in the  $x$ -axis, while the corresponding traffic is depicted in the  $y$ -axis. SR2 has nearly half the traffic of SR206.



**Figure 5.** Traffic volume distribution for each HS for SR2 (a) and for SR206 (b).

### 2.5. The Human Factors Evaluation Tool

The Human Factors Evaluation Tool is a road accident risk assessment tool developed by the researchers of Intelligenz System Transfer GmbH [58] and subsequently promoted and published by PIARC [56]. The tool allows for checking of the presence on the road of safety deficiencies non-compliant with the Human Factors rules. Namely, these are the 4–6 s rule (the road must provide enough time to see and read critical locations), the field of view rule (the road and its surrounding must provide a safe and clear field of view), and the logic rule (the road has to follow a driving logic) [39]. The HFET provides three checklists, one for each of the HF rules, to be filled out. Figure 6 presents an example of a checklist referring to the first HF rule, the 4–6 second rule. This specific checklist is divided into two sub-sections: 1st is “Moderation of transitional areas”, and 2nd is “Perception and visibility”. A critical point is a point on the road that forces the driver to change their driving program (a bend, an intersection, a bus stop, etc.). Column 1 of Figure 6 presents a list of questions referring to a Human Factor demand, i.e., a characteristic that the road should have to allow the correct driving performance in the specific road segment that is being analyzed. Each question should be answered as yes (score 1: the condition is present or satisfied) or no (score 0: the condition is not present, i.e., it is impossible to be analyzed or it is not met). The answer is noted in columns 5 and 6. Column 5 refers to the possible “fulfilment” of the condition (*ACTUAL*), if any. Column 6 (*TARGET*) represents the “presence” or the “desired presence” of the HF demand, depending on the considered road segment. Considering the example in Figure 6, point 2.1b means that the condition is required (target = 1) but not satisfied (actual = 0), while condition 2.2a is not required or it was impossible to analyze (target = 0 and actual = 0) (in this case, there were no curves in the HS). For each item, the worst condition along the inspected road segment was considered, and if a condition was only partially fulfilled, then 0 was annotated in the *ACTUAL* column. Once all the questions have been answered and the respective cells fulfilled, the HFS must be calculated for each sub-section of each checklist (in Figure 6: “Moderation of Transitional Area” and “Perception and Visibility”) and for the entire checklist (“4–6 s Rule Total” in Figure 6).

The HFS is calculated by summing up the values in the *ACTUAL* column and the *TARGET* column and calculating the ratio between *ACTUAL* and *TARGET*, as shown in Equation (1).

$$HFS = \frac{\sum_{i=1}^n ACTUAL_i}{\sum_{i=1}^n TARGET_i} \quad (1)$$

where:

$i$  = number of the considered condition

$n$  = total number of conditions

*ACTUAL* = score in the “*ACTUAL*” column

*TARGET* = score in the “*TARGET*” column

Although only one calculation example for the First Rule is provided in this document, the calculation was done separately for each rule. Moreover, the calculation of the three rules together was also made (i.e., the Total HFS).

The comparison with the accident performance measures was made by considering the result of the Total HFS. The results allow one to classify the HS as having a low, medium, or high accident risk, based on the following criteria [56]:

- Low risk: HFS > 60%
- Medium risk: HFS > 40% but <60%
- High risk: HFS < 40%

For each HS, the HFET evaluation sheets were filled, analyzing each HS through video recordings and notes that were taken during the road inspection. During the HFET application, some additional criteria were adopted to allow its application to segments that may comprise more than one critical point:

- each homogeneous segment was analyzed in both directions; then, a merge of the results was made considering the worst results for each HF demand;
- in processing the checklist of the first HF rule (4–6 s rule), the subsection 1 (“Moderation of transitional area” in Figure 6) was fulfilled for each identified critical point present in each HS, and the worst result was retained in the HFS calculation;
- if one type of critical point was present several times along one segment (for instance, multiple curves), the worst result was taken.

#### 2.6. The HSM Procedure and Accident-Based Performance Measures

The HSM procedure was applied using the IHSDM-HSM Predictive Method software<sup>®</sup>. All available data from each road were included in the model. Two models were considered: the rural two-way two-lane model for the rural stretches of the road and the urban arterial model for the road stretches within the urban environment [17]. The following considerations were made:

- a calibration factor was not available for the Urban arterial model; thus, the calibration factor was considered as 1;
- the calibration factor for the rural two-way two-lane model was derived from the one by Martinelli et al. [59], and it was equal to 0.292;
- the available road characteristics that were included in the calculation are shown in Table 4 (Rural: rural undivided two-lane highway; Urban: urban undivided two-lane arterial).

Two accident-based performance measures were chosen for the analysis. They were the accident frequency and the accident rate. These performance measures were then applied to the expected number of accidents obtained from the HSM procedure. They were calculated as follows:

- Accident frequency (accidents/year): the frequency corresponds to the number of accidents per year in each HS, which is the expected number of accidents derived from the application of the HSM model.
- Accident rate (accidents/(years\*km\*Mvehicles)): this corresponds to the accident density value divided by the value of the average annual traffic that affected the segment during the analysis period. Millions of vehicles ( $10^6$  vehicles) are considered as traffic units. Equation (2) shows how the accident rate was calculated.

$$T = \frac{n}{L \times 365 \times \left(\frac{AADT_m}{10^6}\right)} \quad (2)$$

where

$T$  = accident rate (accidents/(years\*km\*Mvehicles));

$n$  = yearly average number of accidents (accidents/year);

$L$  = segment length (km);

$AADT_m$  = average between the average annual daily traffic value (AADT) of each year of the analysis period (vehicles/day).

Human Factors Man-Road-Interface-Exploration <sup>®</sup> 2017						
Notes:	Road Name	SR206	Direction	North	Evaluator	Andrea Paliotto
	Section Name	36_9			Date	July 2020
	1	2	3	4	5	6
<b>4-6 seconds rule</b>						
	v (km/h)	Length (m)	Length cum. (m)	ACTUAL	TARGET	%
<b>1. Moderation of transitional area</b>						
- manoeuvre section exists?	90	75	75	1	1	
- response section exists? (2-3sec)	90	75	150	1	1	
- anticipation section exists? (2-3sec)	90	75	225	0	1	
- advance warning section exists?	90	75	300	0	1	
<b>Transition zone total</b>				2	4	50%
<b>2. Perception and Visibility</b>						
<b>2.1 critical location visible and clearly identifiable</b>						
a. each critical point is obvious and visible (crossings, driveways, road bends, bus/tram stops, ...)				0	1	
b. Visibility is not restricted by plants, buildings, traffic signs, control devices, roadside furniture, ...)				0	1	
c. roadside furniture/equipment and traffic control devices are clearly visible (traffic signs and signals, markings, safety barriers)				1	1	
d. day: luminance of surface/traffic signs sufficient				1	1	
e. night: lightning and luminance of surface/traffic signs sufficient, retro-reflection of signs and markings is sufficient				1	1	
<b>2.2 curves are visible</b>						
a. curves are visible (at least 6 sec. ahead to the braking section)				0	0	
b. curve is not on/behind a crest				0	0	
c. shoulder and marking of the outer curve are visible				0	0	
d. visibility on the inner curve is not restricted				0	0	
<b>2.3 intersections — visibility triangle from minor road is not obstructed</b>						
a. priority traffic is visible for at least 6 sec. ahead	-	-	-	0	1	
b. intersection is not on or behind a crest				1	1	
c. intersection is better in a sag than on a crest in a hilly terrain				0	0	
d. intersection is not in or after a curve				1	1	
<b>2.4 intersections — minor road: unmistakable right of way</b>						
a. minor road is narrower than the main road				0	0	
b. surface of the main road is of higher quality than the minor road				0	0	
c. lay-out of main and minor road are not similar				0	0	
d. minor road's surface is clearly distinguishable from the main road's surface (e.g. colour variations or different paving material)				0	0	
<b>Perception and visibility total</b>				5	8	63%
<b>4-6 seconds rule total</b>				7	12	58%

Figure 6. Example of HFET sheet fulfilment.

**Table 4.** Available road characteristics included in the model.

Characteristics	Rural	Urban
Horizontal element start location	X	X
Horizontal element end location	X	X
Curve radius	X	X
Curve direction	X	X
Curve side of the road	X	X
Vertical element start location	X	X
Vertical element end location	X	X
Grade	X	X
Lane width	X	X
Overtaking maneuver allowed	X	X
Shoulder width	X	X
Shoulder type	X	X
Design speed	X	
Driveway density	X	
Roadside hazard rating	X	
Lighting	X	X
Automated speed enforcement	X	X
Presence of centerline rumble strip	X	
Presence of median		X
Presence of median barrier		X
Posted speed		X
Speed category		X
Number of driveways		X
Type of driveway		X
Presence of rail highway crossing		X
On-street parking		X

The number of accidents considered in the two previous accident-performance measures is the result of applying the Empirical Bayes procedure to the HSM predictive model. The HSs safety levels, according to accident-based analysis, were defined considering the thresholds' limits adopted by Tuscany Region Administration for the entire network [60]:

- $T_{mean}$ , which is the average of the values of an accident-based performance measure;
- $T_{80}$ , which is the 80th percentile of the values of an accident-based performance measure; and
- $T_{90}$ , which is the 90th percentile of the values of an accident-based performance measure.

The thresholds values considered are shown in Table 5. The given values are referred to the whole road network managed by the Tuscany Region administration and consider only the observed number of accidents. To compare the two approaches considered in the present study (accident-based approach and non-accident-based approach), the HSs were classified as:

- Low risk: performance measure  $< T_{\text{mean}}$ ;
- Medium risk: performance measure  $> T_{\text{mean}}$  and  $< T_{90}$ ;
- High risk: performance measure  $> T_{90}$

**Table 5.** Accident-based performance measures thresholds based on the Tuscany Region network.

Segment Type	Acc. Frequency [Acc./Years]			Acc. Rate [Acc./((Years*km*Mvehicles))]		
	$T_{\text{mean}}$	$T_{80}$	$T_{90}$	$T_{\text{mean}}$	$T_{80}$	$T_{90}$
Roadway	0.16	0.20	0.40	0.16	0.28	0.46
Intersections	0.30	0.40	0.80	0.30	0.53	0.88
Urban	1.25	2.20	3.20	0.50	0.82	1.09
Total	0.44	0.60	1.20	0.28	0.48	0.81

This complies with the meaning of the threshold proposed by the Tuscany Region Administration.

### 2.7. Comparison

The results obtained from the application of the HFET and the accidents-based procedures were compared and discussed. The risk level classification and the HS ranking were compared. The relationship between the results was also tested using a linear correlation. A simple linear correlation was considered because the aim was to evaluate if a relationship exists between the results, not to investigate which function can best represent the relationship. The F-test was selected to consider the relationship between variances. The results were tested considering a significance level of 5%. The correlation analysis was performed considering different data sets: a single dataset, which included the results of both the test roads (“Testing Group” 1—TG1); two distinct datasets, one for each test road (TG2); and three distinct datasets, one for each type of HS (i.e., Roadway Segments, Intersections Segments, and Urban Segments), regardless of the test road (TG3). Table 6 shows the composition of each Testing Group; the second column shows the total number of samples included in each group, while the other columns indicate the number of segments in the sample belonging to SR2 or SR206 and to each segment type, including both the roads.

**Table 6.** Composition of the Testing Groups.

Testing Groups	Samples					
	Total ( <i>n</i> )	Road		Homogeneous Segment Type		
		SR2	SR206	Roadway	Intersections	Urban
TG1	47	22	25	17	22	8
TG2	22	22	-	8	10	4
	25	-	25	9	12	4
TG3	17	8	9	17	-	-
	22	10	12	-	22	-
	8	4	4	-	-	8

## 3. Results

### 3.1. Human Factors Score

The HFS results of SR2 and SR206, deriving from the application of the HFET, are shown in Tables 7 and 8. Table 7 shows the results for every single rule and the total result of the whole rule together, considering the worst results of both directions, as explained

in subchapter 2.5. Table 8 shows the average results for each rule, each road, and each HS type. From the results in Tables 7 and 8, the following can be inferred:

1. Most of the HSs of both the roads have a low score concerning the first rule, which means that the roads do not give enough time to detect, recognize, and correctly react to critical locations.
2. SR2 is characterized by the lowest HFS values, which means that there are more deficiencies concerning the HF demand. It includes two segments (282\_9 and 291\_0) ranked as highly risky. The most frequent deficiencies concern the first HF rule (40%).
3. SR206 has a higher score, confirming that its risk level is lower than SR2. The first HF rule is also the lowest in this case. No HS is of high risk.
4. The urban segments have the lowest mean score in all rules, which means they are the most dangerous ones from the HFS point of view.
5. Roadway segments present fewer safety problems than the intersection and the urban segment.
6. The third rule results are generally higher than or equal to the other results, except for Urban segments, where they are low. This complies with the more complex situation of the urban environment.

**Table 7.** HFS results for each rule, considering both the directions together.

SR 2					SR206				
1	2	3	4	5	1	2	3	4	5
H.S. Name	HFS I Rule	HFS II Rule	HFS III Rule	HFS Total	H.S. Name	HFS I Rule	HFS II Rule	HFS III Rule	HFS Total
281_6	30%	41%	71%	48%	029_6	64%	76%	76%	73%
282_2	38%	68%	65%	59%	030_2	57%	81%	77%	73%
282_7	80%	79%	90%	82%	030_7	75%	82%	87%	83%
282_9	15%	39%	44%	36%	031_1	50%	82%	73%	71%
283_6	30%	71%	72%	67%	031_4	50%	67%	52%	57%
283_9	25%	55%	47%	47%	032_1	40%	56%	41%	45%
284_1	80%	84%	89%	85%	032_8	44%	65%	65%	58%
284_3	56%	71%	81%	72%	033_4	58%	90%	72%	76%
284_6	50%	67%	79%	68%	033_6	40%	87%	79%	72%
285_1	54%	41%	72%	53%	034_2	77%	88%	75%	80%
285_6	31%	54%	52%	48%	034_6	100%	89%	78%	90%
286_5	54%	58%	68%	61%	035_0	24%	52%	44%	42%
287_0	44%	60%	74%	61%	035_9	40%	68%	75%	62%
287_8	15%	67%	50%	48%	036_4	50%	69%	63%	62%
288_6	15%	71%	61%	54%	036_9	40%	72%	50%	57%
289_0	40%	83%	65%	68%	037_4	40%	63%	75%	62%
289_2	31%	58%	38%	44%	038_1	40%	65%	80%	64%
289_9	33%	63%	59%	53%	038_4	33%	72%	57%	58%
290_5	38%	79%	61%	62%	039_1	100%	73%	90%	81%
291_0	19%	42%	39%	34%	039_4	60%	63%	81%	69%
291_7	38%	47%	52%	47%	039_8	100%	73%	90%	82%
292_0	15%	50%	57%	45%	040_1	40%	50%	43%	45%
					040_7	44%	64%	80%	66%
					041_2	90%	80%	87%	84%
					041_4	38%	64%	55%	54%

**Table 8.** Average HFS for each rule.

	First HF Rule	Second HF Rule	Third HF Rule	All HF Rules
Total HSs	48%	67%	67%	62%
SR2	40%	62%	64%	58%
SR206	56%	72%	70%	67%
Roadway	58%	72%	78%	70%
Intersection	44%	65%	65%	60%
Urban	39%	63%	53%	53%

Considering the Human Factors-based approach, it emerged that SR2 has some important safety issues. SR2 presents many changes in the road environment, it includes critical locations that can surprise the driver, and its margins often change and are sometimes ambiguous. SR206 is a “simpler” road, with a long straight and a wide carriageway, where the different elements are clearer and more defined. SR206 has a long straight with a far longer depth of the field of view, but its geometrical monotony is partially compensated by a high number of elements along the margins, which improve the density of the peripheral view, disrupting the monotony and catching the attention of the driver closer to him (without distracting him).

### 3.2. Accident-Based Performance Measures

Table 9 shows the results obtained for the accident-based performance measures (i.e., accident frequency, accident density, and accident rate) derived from the application of the HSM procedure. The HSs are sorted by the value of the accident rate, including both road segments in a single list. The numbers included in the last three columns represent the ranking of each segment based on the value of each accident performance measure. Cell colors represent the level of risk according to the criteria defined in subchapter 2.7: the high risk is red, the medium risk is yellow, and the low risk is green. The thresholds of each risk level refer to the total value, not differentiated by HS type.

**Table 9.** Performance measures results, ranking, and risk level.

Road	HS Name	Acc. Frequency [Acc./y]	Acc. Rate [Acc./(km*Mveh)]	# Acc. Frequency	# Acc. Rate
SR2	291_0	3.75	1.51	1	1
SR2	282_9	1.23	1.17	7	2
SR2	283_9	0.35	1.13	24	3
SR2	281_6	1.00	1.08	9	4
SR206	032_1	2.30	0.76	2	5
SR2	292_0	1.19	0.74	8	6
SR2	282_2	0.52	0.66	17	7
SR2	291_7	0.79	0.59	12	8
SR2	289_9	1.39	0.51	4	9
SR206	031_4	1.35	0.45	5	10
SR206	040_1	1.43	0.43	3	11
SR2	285_6	0.59	0.42	15	12

Table 9. Cont.

Road	HS Name	Acc. Frequency [Acc./y]	Acc. Rate [Acc./km*Mveh]	# Acc. Frequency	# Acc. Rate
SR206	035_9	0.73	0.34	13	13
SR2	287_8	0.41	0.33	21	14
SR2	289_2	0.93	0.29	11	15
SR206	041_4	1.34	0.29	6	16
SR2	289_0	0.08	0.27	33	17
SR206	035_0	0.97	0.25	10	18
SR2	284_6	0.15	0.22	30	19
SR2	290_5	0.50	0.22	18	20
SR206	030_2	0.45	0.21	19	21
SR206	036_9	0.44	0.21	20	22
SR2	285_1	0.15	0.19	29	23
SR206	037_4	0.57	0.19	16	24
SR206	038_4	0.65	0.17	14	25
SR206	032_8	0.37	0.15	22	26
SR2	286_5	0.11	0.14	32	27
SR206	029_6	0.36	0.14	23	28
SR206	030_7	0.21	0.12	27	29
SR206	040_7	0.33	0.12	25	30
SR206	039_1	0.18	0.10	28	31
SR206	036_4	0.30	0.10	26	32
SR2	283_6	0.04	0.09	41	33
SR206	038_1	0.12	0.09	31	34
SR2	288_6	0.05	0.08	39	35
SR2	282_7	0.02	0.08	47	36
SR2	284_3	0.03	0.07	45	37
SR2	284_1	0.02	0.07	46	38
SR2	287_0	0.08	0.06	34	39
SR206	033_4	0.03	0.04	44	40
SR206	041_2	0.04	0.03	43	41
SR206	031_1	0.04	0.03	42	42
SR206	034_2	0.05	0.03	37	43
SR206	034_6	0.05	0.03	38	44
SR206	033_6	0.07	0.03	35	45
SR206	039_8	0.04	0.03	40	46
SR206	039_4	0.06	0.03	36	47

The results are also shown in Figure 7 by histograms. The results of SR2 are presented on the left side and SR206 on the right side. The thresholds  $T_{\text{mean}}$  and  $T_{90}$  are represented respectively with dashed green and red lines.

1. The two considered performance measures lead to different classifications. The exposure value (traffic and segment length) considerably changes the risk ranking. For

instance, the segment SR2\_283\_9 was the third-most critical segment concerning the accident rate, but it was the 24th when considering the accident frequency. Similarly, SR206\_032\_1 was the second segment with the higher accident frequency value, but it was the fifth segment considering accident rate, passing from high risk to medium risk level.

2. The most critical road was SR2. It had four segments above the  $T_{90}$  threshold for accident rate: 281\_6, 282\_9, 283\_9, and 291\_0. The first three were in the hilly terrain and included two narrow bends (first two) and a short segment with many driveways on a bend (SR2\_283\_9). The fourth was in the final part of the analyzed stretch. In this section, the AADT value was higher, and there was a mixture of urban and rural environments.
3. The SR206 has four segments (31\_4, 32\_1, 40\_1, and 41\_4) at high risk considering the accident frequency, while SR2 had only three segments over the  $T_{90}$  thresholds, which were 282\_9, 289\_9, and 291\_0. The segments of SR206 generally “downgraded” to a lower risk if the accident rate was considered due to the higher traffic volume flowing through them.
4. An almost stable ranking was recorded for segments with a low risk of accidents; 26 segments were still low-risk segments for both the performance measures out of a total of 47 segments; six segments were still of medium risk level, and only two segments were still at a high-risk level. The low-risk level group of segments mostly included SR206 segments (16 for SR206 and 10 for SR2).

In conclusion, according to the results of the accident-based approach, SR2 had a high number of high-risk segments considering the accidents rate performance measure, but it was the opposite when considering the accident frequency. Because of the difference underlined in point “a.”, further on, to compare the results of the HFS and the accident-based methodologies, reference will be made only to the accident rate performance measure. The reason is that the accident rate is the performance measure that best fit the HFS, because they both focus only on the infrastructure in-built safety, regardless of exposure (once the index is fixed, the number of accidents will increase as the traffic increase).

### 3.3. Analysis and Comparison of the Results

Results from HFET implementation and accident-based analysis were compared and discussed. The overall results showed more critical conditions for SR2 than SR206.

#### 3.3.1. Risk Level

The HFS and the accident rate results were compared in terms both of risk level and ranking. The ranking considered the two roads together. Table 10 shows the results, representing the risk level by colors and the respective ranking. The HSs are sorted in descending order based on the HFS results.

Table 10 makes it evident that the two approaches globally converge, giving the same response in many cases, but with some differences in the evaluation of the risk level. At the high-risk level, the HFS was more selective: only two HSs were judged as high risk, while the accident-based approach showed four critical segments. In both cases, the high-risk segments belonged to SR2. Fewer differences were found within the medium and low-risk levels, where the two approaches converged in 36 segments. Considering the risk level, 38 out of 47 segments (about 81%) were classified with the same risk level considering the HFS and the accidents rate results.

The most critical HS for both indices was the SR2\_291\_0, which is presented in Figures 8 and 9. In Figure 8 an overview of the HS and the position of accidents that occurred in the segment are presented on a satellite image (picture a). Picture b, c, and d show three different photos taken along the segments. They represent the northbound approach to an intersection and the underpasses (b), the southbound approach to underpasses (c), and the point between the two underpasses southbound (d). A detailed sequence of pictures of the segment taken on the road is presented in Figure 9, considering

the approach to underpasses southbound (from “a” to “e”) and a view of the underpasses northbound (f). The main issues identified are highlighted in the pictures. In this segment, 20 accidents occurred in the analysis period. The partial scores of the three HF rules were 19% for the First Rule, 42% for the Second Rule, and 39% for the Third Rule (see Table 7). There were several overlap problems in this segment. The main issue was represented by the intersection located immediately before an underpass in the north direction, which was completely invisible (picture b in Figure 8). Many side-collision accidents have occurred here. This explains the low score resulting from the First HF rule. Coming from North, two orientation problems occurred in sequence, in correspondence with the existing underpasses that were misaligned. The first one occurred when approaching the first underpass (pictures a, b, c of Figure 9).

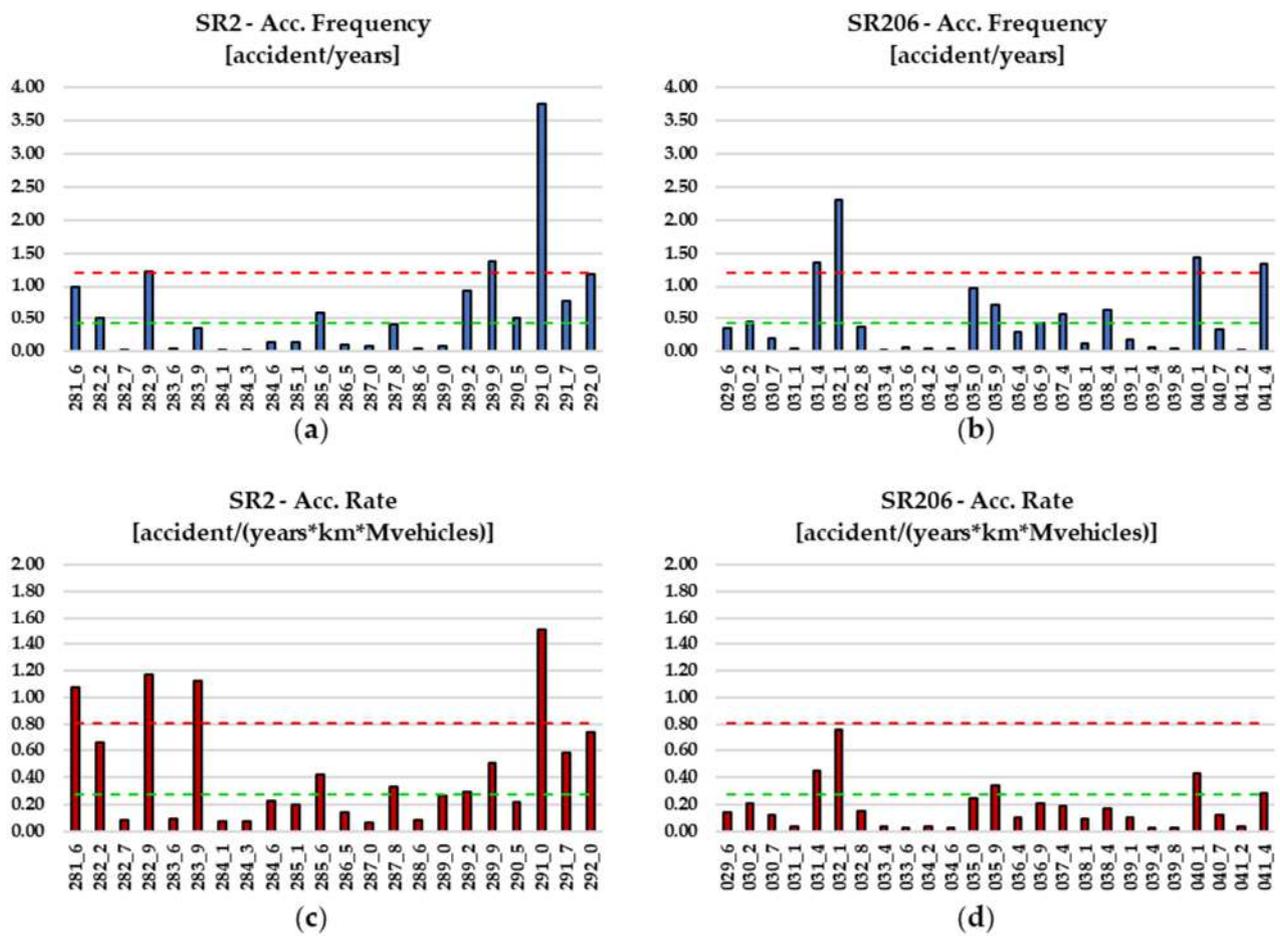
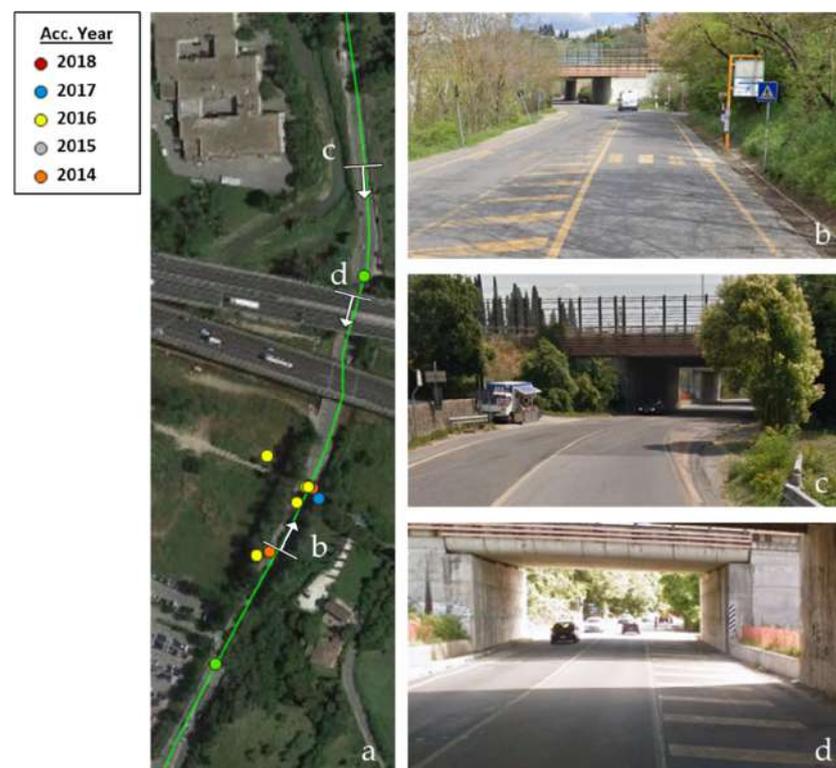


Figure 7. Histogram of the results for each HS, for SR2 accident frequency (a), SR206 accident frequency (b), SR2 accident rate (c), and SR206 accident rate (d).

**Table 10.** Comparison of the risk level classification based on accident rate (all types) and HF.

SR2			SR206		
HS ID	Acc. Rate	HFS	HS ID	Acc. Rate	HFS
291_0	1	1	035_0	18	3
282_9	2	2	032_1	5	6
289_2	15	4	040_1	11	7
292_0	6	5	041_4	16	15
283_9	3	8	031_4	10	18
291_7	8	9	036_9	22	17
285_6	12	10	038_4	25	19
287_8	14	11	032_8	26	20
281_6	4	12	035_9	13	24
289_9	9	13	037_4	24	25
285_1	23	14	036_4	32	27
288_6	35	16	038_1	34	29
282_2	7	21	040_7	30	30
286_5	27	22	039_4	47	33
287_0	39	23	031_1	42	34
290_5	20	26	033_6	45	36
283_6	33	28	030_2	21	37
289_0	17	31	029_6	28	38
284_6	19	32	033_4	40	39
284_3	37	35	034_2	43	40
282_7	36	42	039_1	31	41
284_1	38	46	039_8	46	43
			030_7	29	44
			041_2	41	45
			034_6	44	47



**Figure 8.** SR2\_291\_0 HS with accidents' locations on a satellite image (a) and two pictures of the HS in the northbound direction (b) and one in the southbound direction (c,d).



**Figure 9.** A sequence of pictures approaching the overpasses northbound (a–e) and one picture taken southbound (f).

The second most critical HS was the SR2\_282\_9, which was also ranked second in terms of accident rate, with eight observed accidents in the analysis period. On the left side of Figure 10, a representation of the segment on a satellite image is presented (a). On the right side of the same figure, three pictures are depicted of three critical points of the road southbound (b–d). In the middle of the segment, there is a sharp bend. The stretches preceding the bend from both directions are characterized by low curvature. In the central part of the bend, the shadow produced by the structure of an overpass hides the guidance element (safety barrier) on the external margin of the bend. Five out of eight accidents occurred on the bend, and these accidents were classified as head-on or side collisions. Two of these accidents were caused by drivers driving in the wrong direction and the rest by “distracted or indecisive driving”. This means that the driver could not clearly understand which trajectory they had to follow. The HFS resulted in 15% for the First Rule (the decision distance is missing), 39% for the Second Rule (the driver’s references in the field of view are lacking), and 44% for the Third Rule (the user does not expect the sharp bend) (the results are illustrated in Table 7).

These results underline that the HS has a serious problem of visibility, perception, and comprehension of the road and its elements. The stretches approaching the bend from both directions have a low curvature, thus drivers approach the sharp bend with an inappropriate speed. The bend’s curvature is not well-perceived because of the reduced visibility of lateral guidance. All these conditions can create confusion in drivers, wrong expectations, and underestimation of the risk. A great contribution to the score is given not only by the readability of the bend and its approaching segment, but also by the intersection, which is completely unexpected (see Figure 10—picture b).

In SR206, segment 032\_1 is the most critical segment concerning the accident rate (the fifth when also considering SR2) and the sixth for HFS. In this segment, 14 accidents were observed in the analysis period. As Figure 11 shows, this HS is an Urban Segment, but the urban environment is unclear (houses are far away from the road margins, Figure 11—picture a).

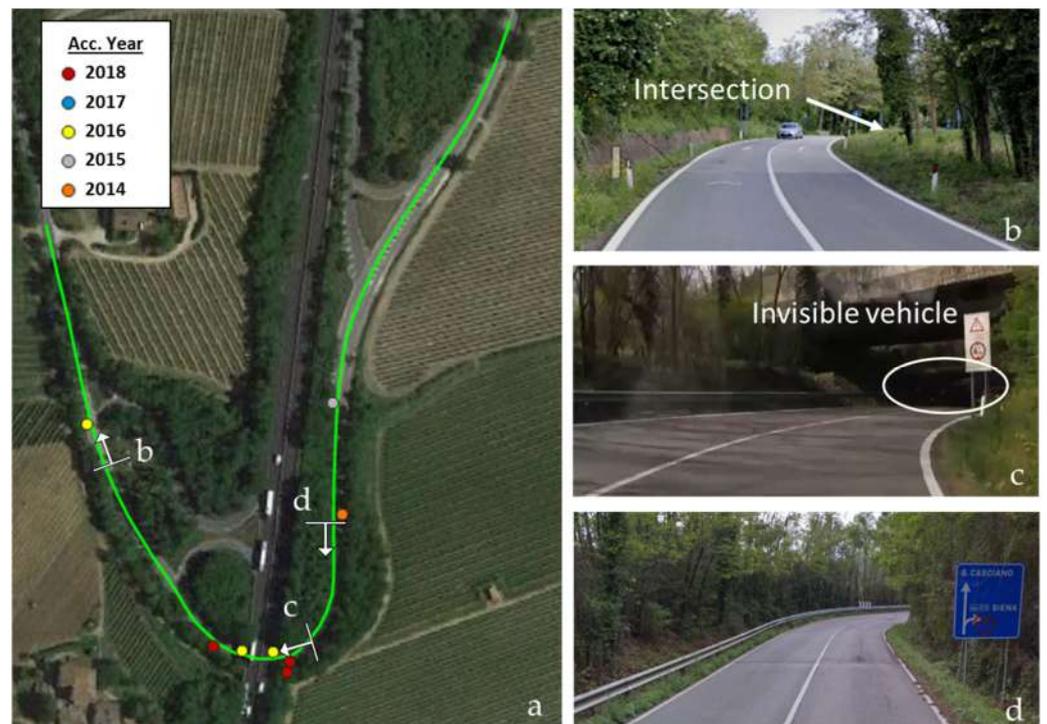


Figure 10. SR2\_282\_9 HS with accidents' locations on a satellite image (a) and three pictures of the HS in the southbound direction (b–d).



Figure 11. SR206\_032\_1 HS with accidents' location on a satellite image (a) and two pictures of the HS in the northbound direction (b,c), and one in the southbound direction, exiting the roundabout (d).

There are driveways, intersections, and also pedestrian crossings (the road divides the residential area from the commercial area). The First Rule of the HF was the one with the lowest score (40%). This was mainly due to the poor visibility of driveways and crosswalks (Figure 11—pictures a and c). The HFS also highlighted some issues concerning the Third Rule (41%), which is the Logic Rule. The urban environment was not perceived, and also, the roundabout and its radius were not well perceived (Figure 11—pictures a and b), causing the driver to be surprised to find those elements.

The segment SR206\_031\_4 was ranked in the medium risk level for HF deficiencies but was burdened by fewer accidents (six accidents were observed in the analysis period). This HS has similar characteristics to the previous one and shows some problems both for the First and Third Rules of HF (50% and 51%, see Table 7). However, the HFS was higher, and the number of accidents was lower than the HS SR206\_032\_1. Figure 12 allows us to clarify this. It shows the approaching segment to the two roundabouts of SR206\_031\_4 (a) and SR206\_032\_1 (b).



**Figure 12.** Approach to roundabout: SR206\_031\_4 (a) and SR206\_032\_1 (b).

The roundabout of SR206\_031\_4 is clearer and more perceivable. The road behind the roundabout is not visible, and the focal point approaching the roundabout is concentrated on the central island; the driver perceives the presence of the critical point (roundabout) in time and reduces the speed. In the roundabout of SR206\_032\_1, the driver's attention is kept by the ascending segment of the road after the roundabout, and they do not concentrate on the roundabout itself, thus reducing to a lesser extent the speed and giving less attention to the vehicles engaging from the roundabout.

In conclusion, the two safety evaluation approaches converge in many cases, but each one presents peculiarity due to the different points of view from which the safety evaluation is done. A brief comment should be made to emphasize once again the proactive value of the HF procedure that analyses the current road situation.

### 3.3.2. Quantitative Relationship between HFS and Accident Rate

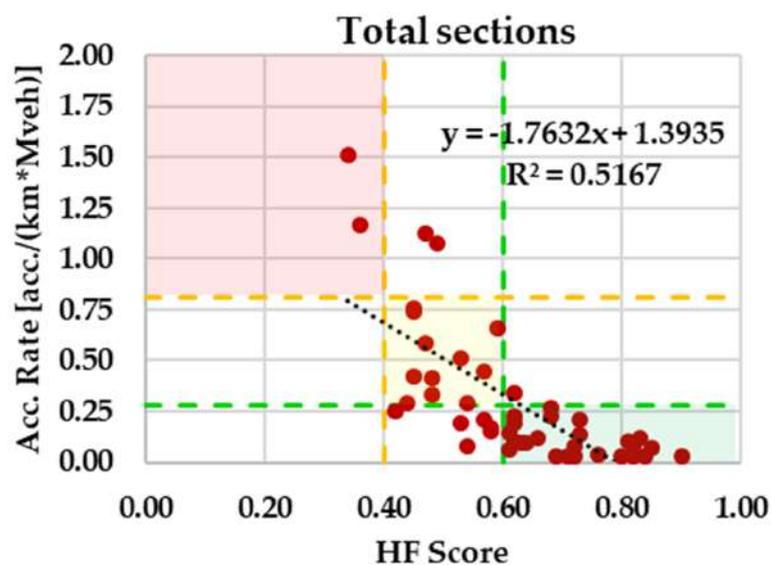
To investigate a trend correlation between the results, a comparison was made in terms of linear correlation, calculating the Pearson linear correlation coefficient,  $r$ , and testing its significance. The correlation was evaluated for the three groups of data (TG1–TG2–TG3), and the results are presented in Table 11. The table shows the testing group composition in the first two columns, the correlation coefficients  $r$  and  $R^2$  in columns 3 and 4, and, in the remaining columns, the significance level ( $\alpha$ , column 5) and the F-test values (column 6 and 7). The test values' column 6 shows the critical value for a significance of 5%, while column 7 shows the calculated actual value. If the actual value is higher than the critical value, the null hypothesis that the distribution occurred by chance can, thus, be rejected. Table 11 shows that for almost all sets, the actual value was higher than the critical value, and thus, the null hypothesis can be rejected. The only set where this condition did not occur was for Urban Segments (TG3). This was due to the quite dispersed distribution and the very low number of elements in the sample (only eight segments).

**Table 11.** Correlation coefficients and statistic values for the HFS and accident rate.

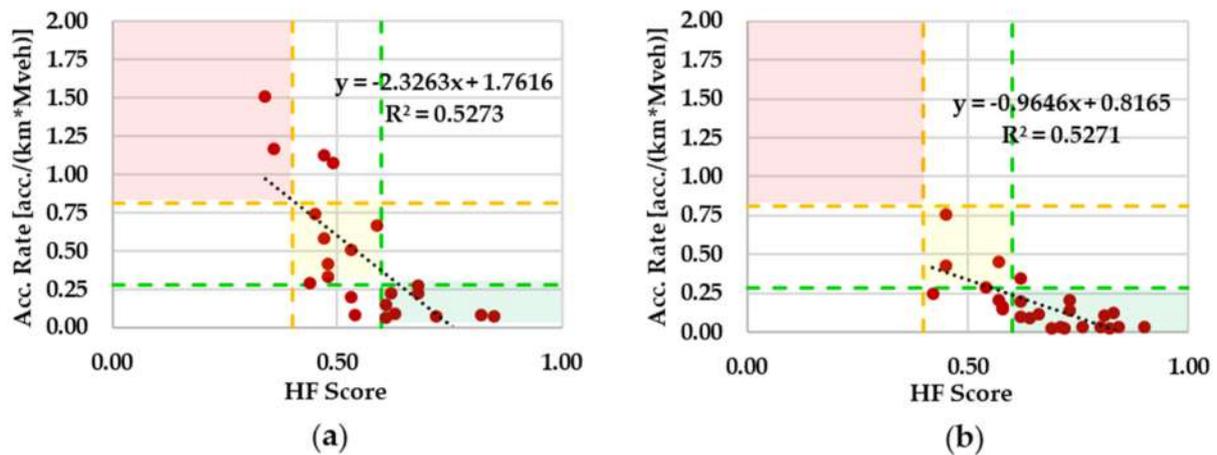
1	2	3	4	5	6	7
Testing Groups		Statistic Results				
		Correlation Coefficients		$\alpha$	F-Test Parameters	
		r	R <sup>2</sup>		F <sub>1, n-2, <math>\alpha/2</math></sub>	F
TG1	Total	−0.719	0.517	5%	2.69	6.94
TG2	SR2	−0.726	0.527	5%	2.09	4.72
	SR206	−0.726	0.527	5%	0.53	5.06
TG3	Roadway Segments	−0.622	0.387	5%	2.13	3.08
	Intersections	−0.756	0.571	5%	2.05	5.16
	Urban Segments	−0.651	0.424	5%	2.45	2.10

The results of r and R<sup>2</sup> show that there was a correlation between the two variables, although it was not very strong. The objective of this analysis was not to produce a predictive model that exactly fitted the data but to demonstrate a correlation between the results and to prove that analyzing Human Factors aspects allows for the identifying dangerous locations.

Figure 13 illustrates the distribution of the HFS values vs the accident rate values for the TG1, including all the HSs of the two test roads together, and Figure 14 shows the HSs divided by road (TG2). The thresholds limits reported in the graphs refer to the total number of segments, not considering the segment type. The dashed green lines represent the thresholds between the medium- and low-risk segments, while the dashed red line represents the thresholds between the high- and medium-risk segments. Figures 13 and 14 allow us to better understand the results of Table 11. There was an inverse relationship between the two variables: the lower the HFS was, the higher the accident rate was.



**Figure 13.** Correlations between HFS and accident rate for TG1.

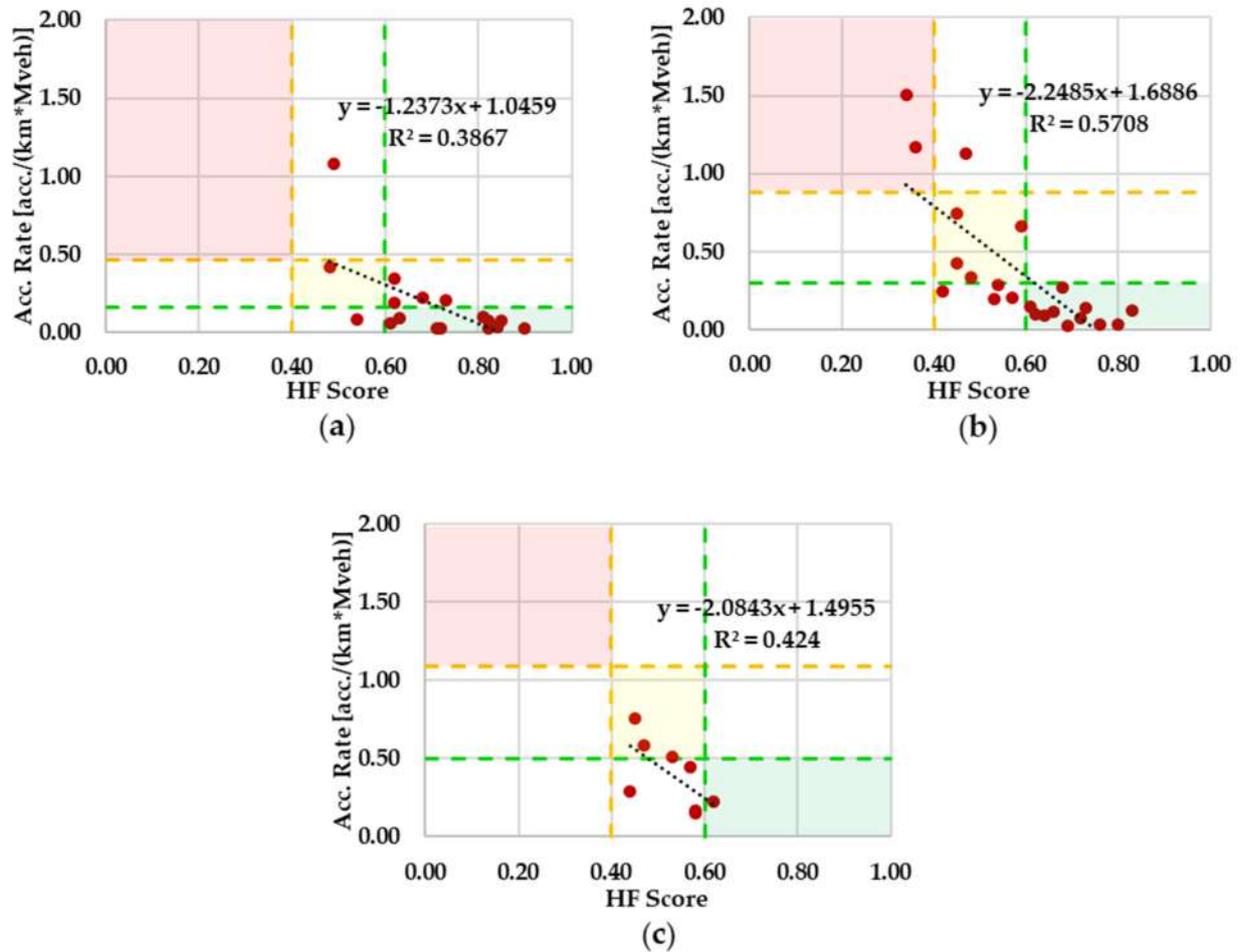


**Figure 14.** Correlations between HFS and accident rate for TG2. The results are presented for SR2 (a) and SR206 (b).

Finally, the relationship between HFS and accident rate was investigated, considering the different types of segments (roadway segments, intersections, and urban stretches, TG3). The results are presented in Figure 15. The threshold limits refer to the specific segments type, e.g., the  $T_{90}$  of an intersection was calculated considering only the segment type “intersection” within the Tuscan Region network. Urban segments were characterized by fewer data. Furthermore, no specific calibration factor for the urban arterial was available. For these reasons, the Urban segment’s result could not be representative.

Looking at the results in Figure 15, a better relationship occurred for Intersections segments rather than Road segments or Urban segments. Furthermore, moving from Roadway segments, which are less complex types of segments (in terms of conflict points and road users flow), to Intersections and Urban segments (more complex segment types) the slope of the linear regression line increased. The conflict probabilities increased in the latter situations, and the number of accidents increased consequently. The general trend found in TG1 and TG2 was confirmed. Thus, it seems that the methodology allowed for identifying of critical segments with enough reliability.

Better results could be obtained if an accident database was available that included all accidents occurring on the road section considered, including those that resulted in property damage only (PDO). The wrong stimuli provided by a low self-explaining road (i.e., a road that does not satisfy the HF demands) can give rise to both serious accidents and PDO accidents. Accidents’ severity is due to the passive safety of the road and accident randomness. The latter is due to the overlapping of random events and an accident’s dynamic (variation of speed at the impact and type of collision). Therefore, a statistical relationship based on a more substantial dataset, including both serious and PDO accidents, should be investigated.



**Figure 15.** Correlations between HFS and accident rate for TG3. The results are presented for Roadway Segments (a), Intersections Segments (b), and Urban Segments (c).

#### 4. Conclusions

Identifying hazardous segments, considering how the road is perceived by drivers, is a fairly new way of analysis. It is not the driver who does not comply with the road but the road that provides misleading information. The inclusion of these aspects in NSSs allows one to considerably improve the effectiveness of the procedure. Moreover, analyzing road safety by employing proactive procedures means not waiting for accidents to occur.

The paper analyzed the effectiveness of a proactive road safety analysis procedure suitable for NSSs, which was based on the analysis of Human Factors aspects. The proposed procedure was based on the use of the HFET proposed by PIARC [56]. The HFET belongs to a proactive investigation methodology that allows for analyzing of the safety performance of a road segment in terms of interaction between the driver and the road environment. The effectiveness of the procedure was tested by comparing the results from the HFET analysis to those of a standard reactive procedure. The latter was based on the application of the HSM predictive model for rural two-lane two-way roads, adjusted for the observed number of accidents using the EB methodology. Both the procedures were applied to two rural two-lane two-way road stretches, for a total length of 23 km.

The outputs of the procedures were compared in terms of risk level and linear correlation. A good correspondence was found in the evaluation. About 81% of the total number of segments analyzed were classified with the same level of risk for both procedures. The linear correlation between the two variables was not strong; thus, a linear function could not clearly explain the relationship between the two variables. However, an

inverse relation was present. Overall, the relationship found confirmed the coherence of the two performance measures. Thus, the used methodology seems able to identify critical road sections.

Some strengths and weaknesses were also found. Adopting a safety screening based on the proposed HFET procedure offers multiple advantages:

- it is a proactive procedure;
- it allows for implementing of NSS, even if reliable accident databases are not available, as mainly occurs in low- and medium-income countries.

On the other hand, one of the main weaknesses of this procedure is that it requires time to be carried out; this may limit its use from RAs. Moreover, the study considered only fatal and injury accidents, and the HFET checklists should be improved in structure and format to be easily applicable.

Consequently, further research should focus on how to reduce the application time and the usability of the HFET. Moreover, the research should be improved by including in the analysis both the severe accidents (as done in the performed study) and the PDO accidents. For further developments, a comprehensive analysis is recommended, considering more roads and including both severe and PDO accidents.

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